

**Transportation Technician  
Qualification Program**

***EMBANKMENT & BASE  
Workbook***





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## **“1998”**

### **Published October 2025**

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## PREFACE

This module is one of a set developed for the Western Alliance for Quality Transportation Construction (WAQTC). WAQTC is an alliance supported by the western state Transportation Departments, along with the Federal Highway Administration (FHWA) and the Western Federal Lands Highway Division (WFLHD) of FHWA. WAQTC's charter includes the following mission.

### MISSION

Provide continuously improving quality in transportation construction.

Through our partnership, we will:

- Promote an atmosphere of trust, cooperation, and communication between government agencies and the private sector
- Respond in a unified and consistent manner to identify quality improvement needs and new technologies that impact the products that we provide,
- Provide a forum to promote uniform test standards,
- Provide highly skilled, knowledgeable materials sampling and testing technicians,
- Provide reciprocity opportunities for qualified testing technicians among Accredited Contributing Members.

### BACKGROUND

There are two significant driving forces behind the development of the WAQTC qualification program. One, there was a trend toward the use of quality control/quality assurance (QC/QA) specifications which included qualification requirements for contractor's QC personnel. Two, Federal regulation on materials sampling and testing (23 CFR 637, *Quality Assurance Procedures for Construction*, published in June 1995) mandated that by June 29, 2000, all testing technicians whose results are used as part of the acceptance decision shall be qualified. In addition, the regulation allows the use of contractor test results to be used as part of the acceptance decision.

### OBJECTIVES

WAQTC's objectives for its Transportation Technician Qualification Program include the following:

- To provide highly skilled, knowledgeable materials sampling and testing technicians.
- To promote uniformity and consistency in testing.
- To provide reciprocity for qualified testing technicians among participating agencies.
- To create a harmonious working atmosphere between public and private employees based upon trust, open communication, and equivalency of qualifications.

Training and qualification of transportation technicians is required for several reasons. It will increase the knowledge of laboratory, production, and field technicians – both industry and agency personnel – and increase the number of available, qualified testers. It will reduce problems associated with test result differences. Regional qualification eliminates the issue of reciprocity among participating agencies and allows qualified testing technicians to cross state lines without needing to be requalified by a different program.

The WAQTC Executive Board

## FOREWORD

This module is one of nine developed to satisfy the training requirements prescribed by Western Alliance for Quality Transportation Construction (WAQTC) for technicians involved in transportation projects. The nine modules cover:

- Aggregate
- Concrete
- Asphalt I
- Asphalt II
- Embankment and Base
- In-place Density
- Embankment and Base/In-place Density
- Self-Consolidating Concrete
- Sampling and Reduction

The modules are based upon AASHTO test methods along with procedures developed by WAQTC. They are narrative in style, illustrated, and include step-by-step instruction. There are review questions at the end of each test procedure, which are intended to reinforce the participants' understanding and help participants prepare for the final written and performance exams. Performance exam check lists are also included. The appendix includes WAQTC Field Operating Procedures (FOPs) in short form.

It is the technician's responsibility to stay current as changes are made to this living document.

The comments and suggestions of every participant are essential to the continued success and high standards of the Transportation Technician Qualification Program. Please take the time to fill out the Course Evaluation Form as the course progresses and hand it in on the last day of class. If you need additional room to fully convey your thoughts, please use the back of the form.

The WAQTC Executive Board



**GUIDANCE FOR COURSE EVALUATION FORM**

The Course Evaluation Form on the following page is very important to the continuing improvement and success of this course. The form is included in each Participant Workbook. During the course introduction, the Instructor will call the participants' attention to the form, its content, and the importance of its thoughtful completion at the end of the course. Participants will be encouraged to keep notes, or write down comments as the class progresses, in order to provide the best possible evaluation. The Instructor will direct participants to write down comments at the end of each day and to make use of the back of the form if more room is needed for comments.

On the last day of the course, just before the written examination, the Instructor will again refer to the form and instruct participants that completion of the form after their last examination is a requirement before leaving. Should the course have more than one Instructor, participants should be directed to list them as A, B, etc., with the Instructor's name beside the letter, and direct their answers in the Instructor Evaluation portion of the form accordingly.



**WESTERN ALLIANCE FOR QUALITY TRANSPORTATION CONSTRUCTION  
COURSE EVALUATION FORM**

The WAQTC Transportation Technician Qualification Program would appreciate your thoughtful completion of all items on this evaluation form. Your comments and constructive suggestions will be an asset in our continuing efforts to improve our course content and presentations.

Course Title: \_\_\_\_\_

Location: \_\_\_\_\_

Dates: \_\_\_\_\_

Your Name (Optional): \_\_\_\_\_

Employer: \_\_\_\_\_

Instructor(s) \_\_\_\_\_

**COURSE CONTENT**

Will the course help you perform your job better and with more understanding? Yes    Maybe    No

Explain: \_\_\_\_\_  
\_\_\_\_\_

Was there an adequate balance between theory, instruction, and hands-on application? Yes    Maybe    No

Explain: \_\_\_\_\_  
\_\_\_\_\_

Did the course prepare you to confidently complete both examinations? Yes    Maybe    No

Explain: \_\_\_\_\_  
\_\_\_\_\_

What was the most beneficial aspect of the course?  
\_\_\_\_\_

What was the least beneficial aspect of the course?  
\_\_\_\_\_

**GENERAL COMMENTS**

General comments on the course, content, materials, presentation method, facility, registration process, etc. Include suggestions for additional Tips!

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**INSTRUCTOR EVALUATION**

Were the objectives of the course, and the instructional and exam approach, clearly explained?	Yes	Maybe	No
--	-----	-------	----

Explain: \_\_\_\_\_

---

Was the information presented in a clear, understandable manner?	Yes	Maybe	No
--	-----	-------	----

Explain: \_\_\_\_\_

---

Did the instructors demonstrate a good knowledge of the subject?	Yes	Maybe	No
--	-----	-------	----

Explain: \_\_\_\_\_

---

Did the instructors create an atmosphere in which to ask questions and hold open discussion?	Yes	Maybe	No
--	-----	-------	----

Explain: \_\_\_\_\_

---

## EMBANKMENT AND BASE

### Learning Objectives

Understanding:

- Quality Assurance (QA) concepts
- Measurements and calculations
- Highway materials terminology
- Safety issues
- Random sampling techniques
- Basics of compaction and density control
- Demonstrating proficiency in the following test procedures:

FOP for AASHTO T 255/T 265

Total Moisture Evaporable Content of Aggregate by Drying,  
Laboratory Determination of Moisture Content of Soils

FOP for AASHTO T 99/T 180

Moisture-Density Relations of Soils Using a 2.5-kg (5.5-lb) Rammer and  
305-mm (12-in.) Drop;  
Moisture-Density Relations of Soils Using a 4.54-kg (10-lb) Rammer and  
457-mm (18-in.) Drop

FOP for AASHTO R 75

Developing Moisture Density Relations (Family of Curves)

FOP for AASHTO T 85

Specific Gravity and Absorption of Coarse Aggregate

The overall goals of this embankment and base course are to understand compaction and density control and to be competent with specific quality control test procedures identified for the Transportation Technician Qualification Program of the Western Alliance for Quality Transportation Construction (WAQTC). Additional studies beyond this course will be required for those desiring greater in-depth knowledge of the theory behind the test procedures included herein.

## Course Outline and Suggested Schedule (Embankment & Base)

### Day One

Welcome  
Introduction of Instructors  
Introduction and Expectations of Participants

WAQTC Mission and TTQP Objectives  
Instructional Objectives for the Course  
Overview of the Course  
Course Evaluation Form

Review of Quality Assurance Concepts

Background in Measurements and Calculations

Random Sampling

Basics of Compaction and Density Control

Total Moisture Content of Aggregate by Drying  
Laboratory Determination of Moisture Content of Soils  
FOP for AASHTO T 255/T 265

Moisture-Density Relations of Soils:  
Using a 2.5-kg (5.5-lb) Rammer and 305-mm (12-in.) Drop  
Using a 4.54-kg (10-lb) Rammer and 457-mm (18-in.) Drop  
FOP for AASHTO T 99/ T 180

Correction for Coarse Particles in the Soil Compaction Test  
Annex to FOP for AASHTO T 99/T 180

Review with Questions and Answers Forum

Afternoon Laboratory Practice

## **Day Two**

Questions from the Previous Day

Specific Gravity and Absorption of Coarse Aggregate  
FOP for AASHTO T 85

Developing Moisture Density Relations (Family of Curves)  
FOP for AASHTO R 75

Review with Questions and Answers Forum

Afternoon Laboratory Practice

## **Day Three**

Start of Exams

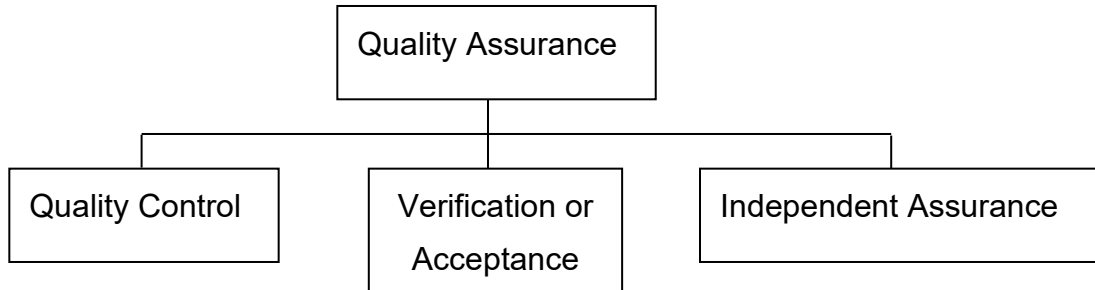
Evaluation



## QUALITY ASSURANCE CONCEPTS

The Federal Highway Administration (FHWA) has established requirements that each State Transportation Department must develop a Quality Assurance (QA) Program that is approved by the FHWA for projects on the National Highway System (NHS). In addition to complying with this requirement, implementing QA specifications in a construction program includes the benefit of improvement of overall quality of highway and bridge construction.

A QA Program may include three separate and distinct parts as illustrated below.



**Quality Assurance (QA)** are those planned and systematic actions necessary to provide confidence that a product or service will satisfy given requirements for quality.

**Quality Control (QC)** are those operational, process control techniques or activities that are performed or conducted to fulfill contract requirements for material and equipment quality. In some states, the constructor is responsible for providing QC sampling and testing, while in other states the STD handles QC. Where the constructor is responsible for QC tests, the results may be used for acceptance only if verified or accepted by additional tests performed by an independent group.

**Verification/Acceptance** consists of the sampling and testing performed to validate QC sampling and testing and, thus, the quality of the product. Verification/Acceptance samples are obtained and tests are performed independently from those involved with QC. Samples taken for QC tests may not be used for Verification/Acceptance testing.

**Independent Assurance (IA)** are those activities that are an unbiased and independent evaluation of all the sampling and testing procedures used in QC and Verification/Acceptance. IA may use a combination of laboratory certification, technician qualification or certification, proficiency samples, or split samples to assure that QC and Verification/Acceptance activities are valid. Agencies may qualify or certify laboratories and technicians, depending on the state in which the work is done.



## BACKGROUND ON MEASUREMENTS AND CALCULATIONS

01

### Introduction

This section provides a background in the mathematical rules and procedures used in making measurements and performing calculations. Topics include:

- Units: Metric vs. English
- Mass vs. Weight
- Balances and Scales
- Rounding
- Significant Figures
- Accuracy and Precision
- Tolerance

Also included is discussion of real-world applications in which the mathematical rules and procedures may not be followed.

02

### Units: Metric vs. English

The bulk of this document uses dual units. Metric units are followed by Imperial, more commonly known as English, units in parentheses. For example: 25 mm (1 in.). Exams are presented in metric or English.

Depending on the situation, some conversions are exact, and some are approximate. One inch is exactly 25.4 mm. If a procedure calls for measuring to the closest 1/4 in., however, 5 mm is close enough. We do not have to say 6.35 mm. That is because 1/4 in. is half way between 1/8 in. and 3/8 in. – or half way between 3.2 and 9.5 mm. Additionally, the tape measure or rule used may have 5 mm marks, but may not have 1 mm marks and certainly will not be graduated in 6 mm increments.

In SI (Le Systeme International d'Unites), the basic unit of mass is the kilogram (kg) and the basic unit of force, which includes weight, is the Newton (N).

03

- Basic units in SI include:
- Length: meter, m
- Mass: kilogram, kg
- Time: second, s
- 

#### SI units

04

<u>Metric</u>	<u>English</u>
25 mm	1 in.
1 kg	2.2 lb
1000 kg/m <sup>3</sup>	62.4 lb/ft <sup>3</sup>
25 MPa	3600 lb/in. <sup>2</sup>

Some approximate conversions

Mass in this document is given in grams (g) or kg. See the section below on “Mass vs. Weight” for further discussion of this topic.

### Mass vs. Weight

The terms mass, force, and weight are often confused. Mass,  $m$ , is a measure of an object’s material makeup, and has no direction. Force,  $F$ , is a measure of a push or pull, and has the direction of the push or pull. Force is equal to mass times acceleration,  $a$ .

$$F = ma$$

Weight,  $W$ , is a special kind of force, caused by gravitational acceleration. It is the force required to suspend or lift a mass against gravity. Weight is equal to mass times the acceleration due to gravity,  $g$ , and is directed toward the center of the earth.

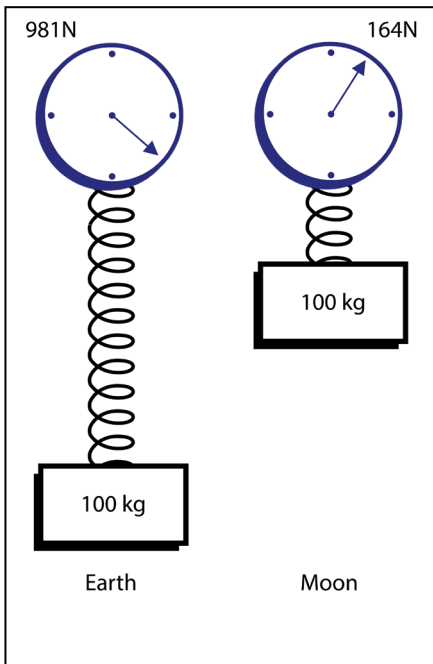
$$W = mg$$

In SI, the basic unit of mass is the kilogram (kg), the units of acceleration are meters per squared second ( $m/s^2$ ), and the unit of force is the Newton (N). Thus a person having a mass of 84 kg subject to the standard acceleration due to gravity, on earth, of  $9.81 m/s^2$  would have a weight of:

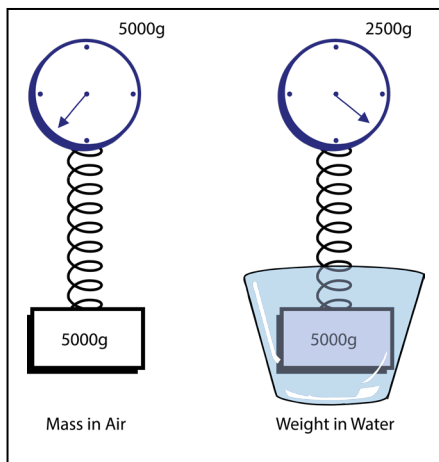
$$W = (84.0 \text{ kg})(9.81 \text{ m/s}^2) = 824 \text{ kg}\cdot\text{m/s}^2 = 824 \text{ N}$$

In the English system, mass can be measured in pounds-mass ( $lb_m$ ), while acceleration is in feet per square second ( $ft/s^2$ ), and force is in pounds-force ( $lb_f$ ). A person weighing 185  $lb_f$  on a scale has a mass of 185  $lb_m$  when subjected to the earth’s standard gravitational pull. If this person were to go to the moon, where the acceleration due to gravity is about one-sixth of what it is on earth, the person’s weight would be about 31  $lb_f$ , while his or her mass would remain 185  $lb_m$ . Mass does not depend on location, but weight does.

While the acceleration due to gravity does vary with position on the earth (latitude and elevation), the variation is not significant except for extremely precise work – the manufacture of electronic memory chips, for example.



Comparison of mass and weight



**Submerged weight**

As discussed above, there are two kinds of pounds,  $lb_m$  and  $lb_f$ . In laboratory measurements of mass, the gram or kilogram is the unit of choice. But, is this mass or force? Technically, it depends on the instrument used, but practically speaking, mass is the result of the measurement. When using a scale, force is being measured – either electronically by the stretching of strain gauges or mechanically by the stretching of a spring or other device. When using a balance, mass is being measured, because the mass of the object is being compared to a known mass built into the balance.

In this document, mass, not weight, is used in test procedures except when determining “weight” in water. When an object is submerged in water (as is done in specific gravity tests), the term weight is used. Technically, what is being measured is the force the object exerts on the balance or scale while the object is submerged in water (or the submerged weight). This force is actually the weight of the object less the weight of the volume of water displaced.

In summary, whenever the common terms “weight” and “weighing” are used, the more appropriate terms “mass” and “determining mass” are usually implied, except in the case of weighing an object submerged in water.

### Balances and Scales

Balances, technically used for mass determinations, and scales, used to weigh items, were discussed briefly above in the section on “Mass vs. Weight.” In field operating procedures, we usually do not differentiate between the two types of instruments. When using either one for a material or object in air, we are determining mass. For those procedures in which the material or object is suspended in water, we are determining weight in water.

AASHTO recognizes two general categories of instruments. Standard analytical balances are used in laboratories. For most field operations, general purpose balances and scales are specified. Specifications for both categories are shown in Tables 1 and 2.

**Table 1**  
**Standard Analytical Balances**

<b>Class</b>	<b>Capacity</b>	<b>Readability and Sensitivity</b>	<b>Accuracy</b>
A	200 g	0.0001 g	0.0002 g
B	200 g	0.001 g	0.002 g
C	1200 g	0.01 g	0.02 g

**Table 2**  
**General Purpose Balances and Scales**

<b>Class</b>	<b>Principal Sample Mass</b>	<b>Readability and Sensitivity</b>	<b>Accuracy</b>
G2	2 kg or less	0.1 g	0.1 g or 0.1 percent
G5	2 kg to 5 kg	1 g	1 g or 0.1 percent
G20	5 kg to 20 kg	5 g	5 g or 0.1 percent
G100	Over 20 kg	20 g	20 g or 0.1 percent

15

### **Rounding**

Numbers are commonly rounded up or down after measurement or calculation. For example, 53.67 would be rounded to 53.7 and 53.43 would be rounded to 53.4, if rounding were required. The first number was rounded up because 53.67 is closer to 53.7 than to 53.6. Likewise, the second number was rounded down because 53.43 is closer to 53.4 than to 53.5. The reasons for rounding are covered in the next section on “Significant Figures.”

If the number being rounded is followed by exactly 5, followed by only zeroes, two possibilities exist. In the more mathematically sound approach, numbers are rounded up or down depending on whether the number to the left of the 5 is odd or even. Thus, 102.25 would be rounded down to 102.2, while 102.35 would be rounded up to 102.4. This procedure avoids the bias that would exist if all numbers ending in 5 were rounded up or all numbers were rounded down. In some calculators, however, all rounding is up. This does result in some bias, or skewing of data, but the significance of the bias may or may not be significant to the calculations at hand.

When rounding numbers that are followed by exactly 5, follow agency guidelines. For the purpose of WAQTC training, if the number being rounded is followed by a 5, the number is increased by 1.

### Significant Figures

- General

16 A general-purpose balance or scale, classified as G20 in AASHTO M 231, has a capacity of 20,000 g and an accuracy requirement of  $\pm 5$  g. A mass of 18,285 g determined with such an instrument could actually range from 18,280 g to 18,290 g. Only four places in the measurement are significant. The fifth (last) place is not significant since it may change.

17 Mathematical rules exist for handling significant figures in different situations.

An example in Metric (**m**) or English(**ft**), when performing addition and subtraction, the number of significant figures in the sum or difference is determined by the least precise input. Consider the three situations shown below:

<u>Situation 1</u>	<u>Situation 2</u>	<u>Situation 3</u>
35.67	143.903	162
+ <u>423.938</u>	- <u>23.6</u>	+33.546
		- <u>.022</u>
= 459.61	= 120.3	= 196
not 459.608	not 120.303	not 195.524

Rules also exist for multiplication and division. These rules, and the rules for mixed operations involving addition, subtraction, multiplication, and/or division, are beyond the scope of these materials. AASHTO covers this topic to a certain extent in the section called “Precision” or “Precision and Bias” included in many test methods, and the reader is directed to those sections if more detail is desired.

18

- Real World Limitations

While the mathematical rules of significant digits have been established, they are not always followed. For example, AASHTO T 176, *Plastic Fines in Graded Aggregates and Soils by the Use of the Sand Equivalent Test*, prescribes a method for rounding and significant digits in conflict with the mathematical rules.

In this procedure, readings and calculated values are always rounded up. A clay reading of 7.94 would be rounded to 8.0 and a sand reading of 3.21 would be rounded to 3.3. The rounded numbers are then used to calculate the Sand Equivalent, which is the ratio of the two numbers multiplied by 100. In this case:

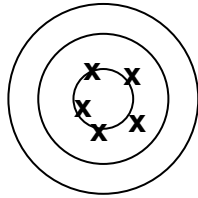
$$\frac{3.3}{8.0} \times 100 = 41.250 \dots$$

rounded to 41.3 and reported as 42

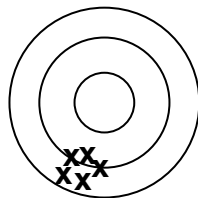
$$\text{Not: } \frac{3.21}{7.94} \times 100 = 40.428 \dots$$

rounded to 40.0 and reported as 40

It is extremely important that engineers and technicians understand the rules of rounding and significant digits just as well as they know procedures called for in standard test methods.



ACCURATE BUT NOT PRECISE,  
SCATTERED



PRECISE BUT NOT ACCURATE,  
BIASED

**Accuracy and Precision**

Although often used interchangeably, the terms accuracy and precision do not mean the same thing. In an engineering sense, accuracy denotes nearness to the truth or some value accepted as the truth, while precision relates to the degree of refinement or repeatability of a measurement.

Two bulls-eye targets are shown to the left. The upper one indicates hits that are scattered and, yet, are very close to the center. The lower one has a tight pattern, but all the shots are biased from the center. The upper one is more accurate, while the lower one is more precise. A biased, but precise, instrument can often be adjusted physically or mathematically to provide reliable single measurements. A scattered, but accurate, instrument can be used if enough measurements are made to provide a valid average.

Consider the measurement of the temperature of boiling water at standard atmospheric pressure by two thermometers. Five readings were taken with each, and the values were averaged.

Thermometer No. 1	Thermometer No. 2
101.2° 214.2°	100.6° 213.1°
101.1° 214.0°	99.2° 210.6°
101.2° 214.2°	98.9° 210.0°
101.1° 214.0°	101.0° 213.8°
101.2° 214.2°	100.3° 212.5°

AVG = 101.2° 214.2°      AVG = 100.0° 212.0°

No. 1 shows very little fluctuation, but is off the known boiling point (100°C or 212°F) by 1.2°C or 2.2°F. No. 2 has an average value equal to the known boiling point, but shows quite a bit of fluctuation. While it might be preferable to use neither thermometer, thermometer No. 1 could be employed if 1.2°C or 2.2°F were subtracted from each measurement. Thermometer No. 2 could be used if enough measurements were made to provide a valid average.

24 Engineering and scientific instruments should be  
calibrated and compared against reference standards  
periodically to assure that measurements are  
accurate. If such checks are not performed, the  
accuracy is uncertain, no matter what the precision.  
25 Calibration of an instrument removes fixed error,  
leaving only random error for concern.

### Tolerance

26 Dimensions of constructed or manufactured objects,  
including laboratory test equipment, cannot be  
specified exactly. Some tolerance must be allowed.  
Thus, procedures for including tolerance in  
addition/subtraction and multiplication/division  
operations must be understood.

- Addition and Subtraction

27 When adding or subtracting two numbers that  
individually have a tolerance, the tolerance of  
the sum or difference is equal to the sum of the  
individual tolerances.

An example in Metric (**m**) or English (**ft**), if the  
distance between two points is made up of two  
parts, one being  $113.361 \pm 0.006$  and the other  
being  $87.242 \pm 0.005$  then the tolerance of the  
sum (or the difference) is:

$$(0.006) + (0.005) = 0.011$$

and the sum would be  $200.603 \pm 0.011$ .

- Multiplication and Division

28 To demonstrate the determination of tolerance  
again in either Metric (**m**) or English (**ft**) for the  
product of two numbers, consider determining  
the area of a rectangle having sides of  $76.254$   
 $\pm 0.009$  and  $34.972 \pm 0.007$ . The percentage  
variations of the two dimensions are:

$$\frac{0.009}{76.254} \times 100 = 0.01\% \quad \frac{0.007}{34.972} \times 100 = 0.02\%$$

The sum of the percentage variations is 0.03  
percent – the variation that is employed in the  
area of the rectangle:

Area =

$$266.8 (m^2 \text{ or } ft^2) = \pm 0.03\% \\ = 2666.8 \pm 0.8 (m^2 \text{ or } ft^2)$$

- Real World Applications

Tolerances are used whenever a product is manufactured. For example, the mold used for determining soil density in AASHTO T 99 has a diameter of  $101.60 \pm 0.41$  mm ( $4.000 \pm 0.016$  in) and a height of  $116.43 \pm 0.13$  mm ( $4.584 \pm 0.005$  in).

Using the smaller of each dimension results in a volume of:

$$\left(\frac{\pi}{4}\right) (101.19 \text{ mm})^2 (116.30 \text{ mm}) \\ = 935,287 \text{ mm}^3 \text{ or } 0.000935 \text{ m}^3$$

$$\left(\frac{\pi}{4}\right) (3.984 \text{ in})^2 (4.579 \text{ in}) \\ = 57.082 \text{ in}^3 \text{ or } 0.0330 \text{ ft}^3$$

Using the larger of each dimension results in a volume of:

$$\left(\frac{\pi}{4}\right) (102.01 \text{ mm})^2 (116.56 \text{ mm}) \\ = 952.631 \text{ mm}^3 \text{ or } 0.000953 \text{ m}^3$$

$$\left(\frac{\pi}{4}\right) (4.016 \text{ in})^2 (4.589 \text{ in}) \\ = 58.130 \text{ in}^3 \text{ or } 0.0336 \text{ ft}^3$$

The average value is  $0.000944 \text{ m}^3$  ( $0.0333$ ), and AASHTO T 99 specifies a volume of:

$$0.000943 \pm 0.000008 \text{ m}^3$$

or a range of

$$0.000935 \text{ to } 0.000951 \text{ m}^3$$

$$0.0333 \pm 0.0003 \text{ ft}^3$$

or a range of

$$0.0330 \text{ to } 0.0336 \text{ ft}^3$$

Because of the variation that can occur, some agencies periodically standardize molds, and make adjustments to calculated density based on those calculations.

### **Summary**

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Mathematics has certain rules and procedures for making measurements and performing calculations that are well established. So are standardized test procedures. Sometimes these agree, but occasionally, they do not. Engineers and technicians must be familiar with both but must follow test procedures in order to obtain valid, comparable results.

## TERMINOLOGY

Many of the terms listed below are defined differently by various agencies or organizations. The definitions of the American Association of State Highway and Transportation Officials (AASHTO) are the ones most commonly used in this document.

**Absorbed water** – Water drawn into a solid by absorption and having physical properties similar to ordinary water.

**Absorption** – The increase in the mass of aggregate due to water being absorbed into the pores of the material, but not including water adhering to the outside surface of the particles, expressed as a percentage of the dry mass.

**Acceptance** – See verification.

**Acceptance program** – All factors that comprise the State Transportation Department's (STD) determination of the quality of the product as specified in the contract requirements. These factors include verification sampling, testing, and inspection and may include results of quality control sampling and testing.

**Admixture** – Material other than water, cement, and aggregates in Portland cement concrete (PCC).

**Adsorbed water** – Water attached to the surface of a solid by electrochemical forces and having physical properties substantially different from ordinary water.

**Aggregate** – Hard granular material of mineral composition, including sand, gravel, slag, or crushed stone, used in roadway base and in Portland cement concrete (PCC) and asphalt mixtures.

- **Coarse aggregate** – Aggregate retained on or above the No. 4 (4.75 mm) sieve.
- **Coarse-graded aggregate** – Aggregate having a predominance of coarse sizes.
- **Dense-graded aggregate** – Aggregate having a particle size distribution such that voids occupy a relatively small percentage of the total volume.
- **Fine aggregate** – Aggregate passing the No. 4 (4.75 mm) sieve.
- **Fine-graded aggregate** – Aggregate having a predominance of fine sizes.
- **Mineral filler** – A fine mineral product at least 70 percent of which passes a No. 200 (75  $\mu$ m) sieve.
- **Open-graded gap-graded aggregate** – Aggregate having a particle size distribution such that voids occupy a relatively large percentage of the total volume.
- **Well-Graded Aggregate** – Aggregate having an even distribution of particle sizes.

**Aggregate storage bins** – Bins that store aggregate for feeding material to the dryer in an asphalt mixture plant in substantially the same proportion as required in the finished mix.

**Agitation** – Provision of gentle motion in Portland cement concrete (PCC) sufficient to prevent segregation and loss of plasticity.

**Air voids ( $V_a$ )** – Total volume of the small air pockets between coated aggregate particles in asphalt mixtures; expressed as a percentage of the bulk volume of the compacted paving mixture.

**Ambient temperature** – Temperature of the surrounding air

**Angular aggregate** – Aggregate possessing well-defined edges at the intersection of roughly planar faces.

**Apparent specific gravity ( $G_{sa}$ )** – The ratio of the mass, in air, of a volume of the impermeable portion of aggregate to the mass of an equal volume of water at a stated temperature.

**Asphalt** – A dark brown to black cementitious material in which the predominate constituents are bitumens occurring in nature or obtained through petroleum processing. Asphalt is a constituent of most crude petroleum.

**Asphalt emulsion** – A mixture of asphalt binder and water.

**Asphalt binder** – An asphalt specially prepared in quality and consistency for use in the manufacture of asphalt mixtures.

**Asphalt mixtures** – High quality, thoroughly controlled mix of aggregate and asphalt binder.

- **Hot mix asphalt (HMA)** – Asphalt mixtures of well-graded aggregate and asphalt binder that are mixed and placed at high temperatures.
- **Stone matrix asphalt (SMA)** – A gap-graded hot asphalt mixture that is designed to maximize deformation (rutting) resistance and durability by using a structural basis of stone-on-stone contact.
- **Warm mix asphalt (WMA)** – Asphalt mixtures that, due to a variety of technologies, are mixed and placed at relatively lower temperatures than HMA.

**Automatic cycling control** – A control system in which the opening and closing of the weigh hopper discharge gate, the bituminous discharge valve, and the pugmill discharge gate are actuated by means of automatic mechanical or electronic devices without manual control. The system includes preset timing of dry and wet mixing cycles.

**Automatic dryer control** – A control system that automatically maintains the temperature of aggregates discharged from the dryer.

**Automatic proportioning control** – A control system in which proportions of the aggregate and asphalt binder fractions are controlled by means of gates or valves that are opened and closed by means of automatic mechanical or electronic devices without manual control.

**Bag (of cement)** – 94 lb of Portland cement (Approximately 1 ft<sup>3</sup> of bulk cement)

**Base** – A layer of selected material constructed on top of subgrade or subbase and below the paving on a roadway.

**Bias** – The offset or skewing of data or information away from its true or accurate position as the result of systematic error.

**Binder** – Asphalt binder or modified asphalt binder that binds the aggregate particles into a dense mass.

**Bleed** – Occurs in concrete when coarse aggregate tends to settle down and free water rises to the surface.

**Boulders** – Rock fragment, often rounded, with an average dimension larger than 300 mm (12 in.).

**Bulk specific gravity** – The ratio of the mass, in air, of a volume of aggregate ( $G_{sa}$ ) or compacted asphalt mixture ( $G_{mb}$ ) (including the permeable and impermeable voids in the particles, but not including the voids between particles) to the mass of an equal volume of water at a stated temperature.

**Bulk specific gravity (SSD)** – The ratio of the mass, in air, of a volume of aggregate ( $G_{sa SSD}$ ) or compacted asphalt mixtures ( $G_{mb SSD}$ ), including the mass of water within the voids (but not including the voids between particles), to the mass of an equal volume of water at a stated temperature. (See saturated surface dry.)

**Cementitious Materials** – cement and pozzolans used in concrete such as: Portland cement, fly ash, silica fume, and blast-furnace slag.

**Clay** – Fine-grained soil that exhibits plasticity over a range of water contents, and that exhibits considerable strength when dry, also, that portion of the soil finer than 2  $\mu$ m.

**Cobble** – Rock fragment, often rounded, with an average dimension between 75 and 300 mm (3 and 12 in.).

**Cohesionless soil** – Soil with little or no strength when dry and unconfined or when submerged, such as sand

**Cohesive soil** – Soil with considerable strength when dry and that has significant cohesion when unconfined or submerged.

**Compaction** – Densification of a soil or asphalt mixtures by mechanical means.

**Compaction curve (Proctor curve or moisture-density curve)** – The curve showing the relationship between the dry unit weight or density and the water content of a soil for a given compactive effort.

**Compaction test (moisture-density test)** – Laboratory compaction procedure in which a soil of known water content is placed in a specified manner into a mold of given dimensions, subjected to a compactive effort of controlled magnitude, and the resulting density determined.

**Compressibility** – Property of a soil or rock relating to susceptibility to decrease in volume when subject to load.

**Constant mass** – The state at which a mass does not change more than a given percent, after additional drying for a defined time interval, at a required temperature.

**Constructor** – The builder of a project. The individual or entity responsible for performing and completing the construction of a project required by the contract documents. Often called a contractor, since this individual or entity contracts with the owner.

**Cutback asphalt** – Asphalt binder that has been modified by blending with a chemical solvent.

**Crusher-run** – The total unscreened product of a stone crusher.

**Delivery tolerances** – Permissible variations from the desired proportions of aggregate and asphalt binder delivered to the pugmill.

**Density** – The ratio of mass to volume of a substance. Usually expressed in lb/ft<sup>3</sup> (kg/m<sup>3</sup>).

**Design professional** – The designer of a project. This individual or entity may provide services relating to the planning, design, and construction of a project, possibly including materials testing and construction inspection. Sometimes called a “contractor,” since this individual or entity contracts with the owner.

**Dryer** – An apparatus that dries aggregate and heats it to specified temperatures.

**Dry mix time** – The time interval between introduction of aggregate into the pugmill and the addition of asphalt binder.

**Durability** – The property of concrete that describes its ability to resist disintegration by weathering and traffic. Included under weathering are changes in the pavement and aggregate due to the action of water, including freezing and thawing.

**Dust Proportion – DP (Dust to Effective (asphalt) Binder Ratio)** – The percent passing the No. 200 sieve divided by the percent of effective asphalt binder.

**Effective specific gravity ( $G_{se}$ )** – The ratio of the mass in air of a unit volume of a permeable material (excluding voids permeable to asphalt binder) at a stated temperature to

the mass in air (of equal density) of an equal volume of gas-free distilled water at a stated temperature.

**Effective diameter (effective size)** –  $D_{10}$ , particle diameter corresponding to 10 percent finer or passing.

**Embankment** – Controlled, compacted material between the subgrade and subbase or base in a roadway.

**End-result specifications** – Specifications that require the Constructor to take the entire responsibility for supplying a product or an item of construction. The Owner's (the highway agency's) responsibility is to either accept or reject the final product or to apply a price adjustment that is commensurate with the degree of compliance with the specifications. Sometimes called performance specifications, although considered differently in highway work. (See performance specifications.)

**Family of curves** – a group of soil moisture-density relationships (curves) determined using AASHTO T 99 or T 180, which reveal certain similarities and trends characteristic of the soil type and source.

**Field operating procedure (FOP)** – Procedure used in field testing on a construction site or in a field laboratory. (Based on AASHTO or NAQTC test methods.)

**Fineness modulus** – A factor equal to the sum of the cumulative percentages of aggregate retained on certain sieves divided by 100; the sieves are 150, 75, 37.5, 19.0, 9.5, 4.75, 2.36, 1.18, 0.60, 0.30, and 0.15 mm. Used in the design of concrete mixes. The lower the fineness modulus, the more water/cement paste that is needed to coat the aggregate.

**Fines** – Portion of a soil or aggregate finer than a 75  $\mu\text{m}$  (No. 200) sieve. Also silts and clays.

**Fractured criteria** – The specified requirement for fractured particles determined by each agency.

**Fractured face** – An angular, rough, or broken surface of an aggregate particle created by crushing or by other means. A face is considered a "fractured face" whenever one-half or more of the projected area, when viewed normal to that face, is fractured with sharp and well-defined edges. This excludes small nicks.

**Fractured particle** – A particle of aggregate having at least the minimum number of fractured faces specified.

**Free water** – Water on aggregate available for reaction with hydraulic cement. Mathematically, the difference between total moisture content and absorbed moisture content.

**Glacial till** – Material deposited by glaciation, usually composed of a wide range of particle sizes, which has not been subjected to the sorting action of water.

**Gradation (grain-size distribution)** – The proportions by mass of a soil or fragmented rock distributed by particle size.

**Gradation analysis (grain size analysis or sieve analysis)** – The process of determining grain-size distribution by separation of sieves with different size openings.

**Halo** – A concentration of mortar that can form at the perimeter of the slump flow patty.

**High-range water-reducer (HRWR)** – A concrete admixture that can reduce the quantity of mixing water required to produce concrete by more than 12 percent, while maintaining a certain level of consistency in slump.

**Hot aggregate storage bins** – Bins that store heated and separated aggregate before final proportioning into the mixer.

**Hot Mix Asphalt (HMA) batch plant** – A manufacturing facility for producing asphalt mixture that proportions aggregate by weight and asphalt by weight or volume.

**HMA continuous mix plant** – A manufacturing facility for producing asphalt mixture that proportions aggregate and asphalt binder by a continuous volumetric proportioning system without specific batch intervals.

**Hydraulic cement** – Cement that sets and hardens by chemical reaction with water.

**Independent assurance** – Unbiased and independent evaluation of all the sampling and testing procedures, equipment, and technicians involved with Quality Control (QC) and Verification/Acceptance.

**In situ** – Rock or soil in its natural formation or deposit.

**J-Ring** – a rigid ring made of steel connecting 100 mm (4 in.) vertical smooth bars used in testing the passing ability of SCC.

**Liquid limit** – Moisture content corresponding to the boundary between the liquid and plastic states.

**Loam** – A mixture of sand, silt or clay, or a combination thereof, with organic matter.

**Lot** – A quantity of material to be controlled. It may represent a specified mass, a specified number of truckloads, or a specified time period during production.

**Manual proportioning control** – A control system in which proportions of the aggregate and asphalt binder fractions are controlled by means of gates or valves that are opened and closed by manual means. The system may or may not include power assisted devices in the actuation of gate and valve opening and closing.

**Materials and methods specifications** – Also called prescriptive specifications. Specifications that direct the Constructor to use specified materials in definite proportions and specific types of equipment and methods to place the material.

**Maximum size** – One sieve larger than nominal maximum size.

**Mesh** – The square opening of a sieve.

**Moisture content** – The ratio, expressed as a percentage, of the mass of water in a material to the dry mass of the material.

**Nominal maximum size** – One sieve larger than the first sieve to retain more than 10 percent of the material using an agency specified set of sieves based on cumulative percent retained. Where large gaps in specification sieves exist, intermediate sieve(s) may be inserted to determine nominal maximum size.

*Note:* The first sieve to normally retain more than 10 percent of the material usually is the second sieve in the stack but may be the third sieve.

**Nuclear gauge** – Instruments used to measure in-place density, moisture content, or asphalt binder content through the measurement of nuclear emissions.

**Optimum moisture content (optimum water content)** – The water content at which a soil can be compacted to a maximum dry density by a given compactive effort.

**Organic soil** – Soil with a high organic content.

**Owner** – The organization that conceives of and eventually operates and maintains a project. A State Transportation Departments (STD) is an Owner.

**Passing ability** – An indication of the ability of the SCC to flow around and between reinforcement without blocking.

**Paste** – Mix of water and hydraulic cement that binds aggregate in Portland cement concrete (PCC).

**Penetration** – The consistency of a bituminous material, expressed as the distance in tenths of a millimeter (0.1 mm) that a standard needle vertically penetrates a sample of the material under specified conditions of loading, time, and temperature.

**Percent of Absorbed (asphalt) Binder ( $P_{ba}$ )** – The total percent of the asphalt binder that is absorbed into the aggregate, expressed as a percentage of the mass of aggregate rather than as a percentage of the total mass of the mixture. This portion of the asphalt binder content does not contribute to the performance of the mix.

**Percent aggregate (stone) ( $P_s$ )** – The percent aggregate (stone) content, expressed as a percentage of the total mass of the sample.

**Percent of Effective (asphalt) Binder ( $P_{be}$ )** – The total asphalt binder content of a paving mixture minus the portion of asphalt binder that is lost by absorption into the aggregate

particles, expressed as a percentage of the mass of aggregate. It is the portion of the asphalt binder content that remains as a coating on the outside of the aggregate particles.

**Percent compaction** – The ratio of density of a soil, aggregate, or asphalt mixtures in the field to a maximum density determined by a standard compaction test, expressed as a percentage.

**Performance specifications** – Specifications that describe how the finished product should perform. For highways, performance is typically described in terms of changes over time in physical condition of the surface and its response to load, or in terms of the cumulative traffic required to bring the pavement to a condition defined as “failure.” Specifications containing warranty/guarantee clauses are a form of performance specifications.

**Plant screens** – Screens located between the dryer and hot aggregate storage bins that separate the heated aggregates by size.

**Plastic limit** – Moisture content corresponding to the boundary between the plastic and the semisolid states.

**Plasticity** – Property of a material to continue to deform indefinitely while sustaining a constant stress.

**Plasticity index** – Numerical difference between the liquid limit and the plastic limit and, thus, the range of water content over which the soil is plastic.

**Portland cement** – Hydraulic cement produced by pulverizing Portland cement clinker.

**Portland cement concrete (PCC)** – A controlled mix of aggregate, Portland cement, and water, and possibly other admixtures.

**PCC batch plant** – A manufacturing facility for producing Portland cement concrete.

**Prescriptive specifications** – See Materials and Methods specification.

**Proficiency samples** – Homogeneous samples that are distributed and tested by two or more laboratories. The test results are compared to assure that the laboratories are obtaining the same results.

**Pugmill** – A shaft mixer designed to mix aggregate and cement.

**Quality assurance** – Planned and systematic actions necessary to provide confidence that a product or service will satisfy given requirements for quality. The overall system for providing quality in a constructed project, including Quality Control (QC), Verification/Acceptance, and Independent Assurance (IA).

**Quality assurance specifications** – Also called QC/QA specifications. A combination of end-result (performance) specifications and materials and methods (prescriptive)

specifications. The Constructor is responsible for quality control, and the Owner (highway agency) is responsible for acceptance of the product.

**Quality control (QC)** – Operational, process control techniques or activities that are performed or conducted to fulfill contract requirements for material or equipment quality.

**Random sampling** – Procedure for obtaining non-biased, representative samples.

**Recycled (reclaimed) asphalt materials** – Recycled asphalt pavement (RAP) and recycled asphalt shingles (RAS) used as a component in asphalt mixtures.

**Sand** – Particles of rock passing the No. 4 (4.75 mm) sieve and retained on the No. 200 (75  $\mu$ m) sieve.

**Saturated surface dry (SSD)** – Condition of an aggregate particle, asphalt mixtures or Portland cement concrete (PCC) core, or other porous solid when the permeable voids are filled with water, but no water is present on exposed surfaces. (See bulk specific gravity.)

**Sectoring** – An alternate method for further reducing quarters of a sample of material.

**Self-Consolidating Concrete (SCC)** – A highly flowable non-segregating concrete mix that spreads into place and is able to flow and fill all corners of the formwork, even in the presence of congested reinforcement by means of its own mass with no mechanical vibration.

**Segregation** – The separation of aggregate by size resulting in a non-uniform material.

**Sieve** – Laboratory apparatus consisting of wire mesh with square openings, usually in circular or rectangular frames.

**Silt** – Material passing the (75  $\mu$ m) sieve that is non-plastic or very slightly plastic, and that exhibits little or no strength when dry and unconfined. Also, that portion of the soil finer than 75  $\mu$ m and coarser than 2  $\mu$ m.

**Slump** – Measurement related to the workability of concrete.

**Slump flow** – Assesses the horizontal free flow, filling ability of self-compacting concrete in the absence of obstructions and may give some indication of resistance to segregation. It does not indicate the ability of the SCC to pass between reinforcement without blocking.

**Smartly** – Quickly and usually done with force.

**Soil** – Sediments or unconsolidated accumulations of solid particles produced by the physical and chemical disintegration of rocks, and which may or may not contain organic matter.

**Specific gravity** – The ratio of the mass of a volume of a material to the mass of an equal volume of water at a stated temperature.

- **G<sub>mm</sub>** – theoretical maximum specific gravity (Gravity<sub>mix max</sub>)  
The ratio of the mass of a given volume of asphalt mixtures with no air voids to the mass of an equal volume of water, both at a stated temperature.
- **G<sub>mb</sub>** – measured bulk specific gravity (Gravity<sub>mix bulk</sub>)  
The ratio of the mass, in air, of a volume of compacted asphalt mixture (including the permeable and impermeable voids in the particles but not including the voids between particles) to the mass of an equal volume of water at a stated temperature.
- **G<sub>sb</sub>** – oven-dry bulk specific gravity of aggregate (Gravity<sub>stone bulk</sub>)  
The ratio of the mass, in air, of a volume of aggregate (including the permeable and impermeable voids in the particles but not including the voids between particles) to the mass of an equal volume of water at a stated temperature.
- **G<sub>sa</sub>** – apparent specific gravity of aggregate (Gravity<sub>stone apparent</sub>)  
The ratio of the mass, in air, of a volume of the impermeable portion of aggregate to the mass of an equal volume of water at a stated temperature.
- **G<sub>se</sub>** – effective specific gravity of aggregate (Gravity<sub>stone effective</sub>)  
The ratio of the mass in air of a unit volume of a permeable material (excluding voids permeable to asphalt binder) at a stated temperature to the mass in air (of equal density) of an equal volume of gas-free distilled water at a stated temperature.
- **G<sub>b</sub>** – specific gravity of the binder (Gravity<sub>binder</sub>)  
The ratio of the mass of a volume of asphalt binder to the mass of an equal volume of water at a stated temperature.

**Spine** – smooth line extending through the point of maximum density and optimum moisture content of a family of moisture-density curves.

**Stability** – The ability of an asphalt mixture to resist deformation from imposed loads. Stability is dependent upon internal friction, cohesion, temperature, and rate of loading.

**Static segregation** - The tendency for coarse aggregate to separate from the sand-cement mortar in SCC.

**Stratified random sampling** – Procedure for obtaining non-biased, representative samples in which the established lot size is divided into equally-sized sublots.

**Subbase** – A layer of selected material constructed between the subgrade and the base course in a flexible asphalt mixture roadway, or between the subgrade and Portland cement concrete (PCC) pavement in a rigid PCC roadway.

**Subgrade** – Natural soil prepared and compacted to support a structure or roadway pavement.

**Sublot** – A segment of a lot chosen to represent the total lot.

**Superpave™** – Superpave™ (Superior Performing Asphalt Pavement) is a trademark of the Strategic Highway Research Program (SHRP). Superpave™ is a product of the SHRP asphalt research. The Superpave™ system incorporates performance-based asphalt materials characterization with design environmental conditions to improve performance by controlling rutting, low temperature cracking and fatigue cracking. The three major components of Superpave™ are the asphalt binder specification, the mix design and analysis system, and a computer software system.

**Theoretical maximum specific gravity ( $G_{mm}$ )** – The ratio of the mass of a given volume of asphalt mixtures with no air voids to the mass of an equal volume of water, both at a stated temperature.

**Topsoil** – Surface soil, usually containing organic matter.

**Uniformity coefficient** –  $C_u$ , a value employed to quantify how uniform or well-graded an aggregate is:  $C_u = D_{60}/D_{10}$ . 60 percent of the aggregate, by mass, has a diameter smaller than  $D_{60}$  and 10 percent of the aggregate, by mass, has a diameter smaller than  $D_{10}$ .

**Unit weight** – The ratio of weight to volume of a substance. The term “density” is more commonly used.

**μm** – Micrometer (micron), a millionth of a meter. Used as a measurement for sieve size.

**Vendor** – Supplier of project-produced material that is other than the constructor.

**Verification** – Process of sampling and testing performed to validate Quality Control (QC) sampling and testing and, thus, the quality of the product. Sometimes called Acceptance.

**Viscosity modifying admixture** – chemical additives used in concrete to control flow properties and enhance durability.

**Visual stability index (VSI)** – The Visual Stability Index (VSI) is used to assess the stability of SCC. The stability (or segregation resistance) of an SCC mixture is the ability of the mixture to remain homogeneous during transport, during placement, and after placement. The VSI determination is useful for quality control and consistency testing.

**Void in the mineral aggregate (VMA)** – The volume of inter-granular void space between aggregate particles of compacted asphalt mixtures that includes air and asphalt binder; expressed as a percentage of the bulk volume of the compacted paving mixture.

**Void filled with asphalt (VFA)** – The portion of the void in the mineral aggregate (VMA) that contains asphalt binder; expressed as a percentage of the bulk volume of mix or the VMA.

**Wet mixing period** – The time interval between the beginning of application of asphalt binder and the opening of the mixer gate.

**Zero air voids curve (saturation curve)** – Curve showing the zero air voids density as a function of water content.

## SAFETY

The procedures included in this manual may involve hazardous materials, operations, and equipment. The procedures do not address all of the safety issues associated with their use. It is the responsibility of the employer to assess workplace hazards and to determine whether personal protective equipment (PPE) must be used. PPE must meet applicable American National Standards Institute (ANSI) standards and be properly used and maintained. The employer must establish appropriate safety and health practices, in compliance with applicable state and federal laws, for these procedures and associated job site hazards. Hazardous materials must be addressed in a Hazard Communication program, and Material Safety Data Sheets (MSDS) must be obtained and available to workers. Supervisors and employees should be aware of job site hazards and comply with their employer's safety and health program. The following table identifies some areas that may affect individuals performing the procedures in this manual.

<b>Body Part Affected</b>	<b>Potential Hazards</b>	<b>PPE/Procedures That May Be Appropriate</b>
Head	Falling or fixed overhead objects; electrical shock	Hard hat or other protective helmet
Eyes and Face	Flying objects, radiation, molten metal, chemicals	Safety glasses, goggles, face shields; prescription or filter lenses
Ears	Noise	Ear plugs, earmuffs
Respiratory System	Inhalation of dusts, chemicals; O <sub>2</sub> deficiency	Properly fit and used respiratory protection consistent with the hazard
Skin	Chemicals including cement; heat	Appropriate chemical or heat resistant gloves, long-sleeve shirts, coveralls
Mouth, digestive system	Ingestion of toxic materials	Disposable or washable gloves, coveralls; personal hygiene
Hands	Physical injury (pinch, cut, puncture), chemicals	Appropriate gloves for physical hazards and compatible with chemicals present
Feet	Falling, sharp objects; slippery surfaces, chemicals	Safety shoes or boots (steel toed, steel shank); traction soles; rubber boots – chemicals, wet conditions
Joints, muscles, tendons	Lifting, bending, twisting, repetitive motions	Proper training and procedures; procedure modifications
Body/Torso	Falls; Burial	Fall protection; trench sloping or shoring
Miscellaneous	Traffic	Visibility, awareness, communication; driver training, safety awareness
Whole body	Radiation	Radiation safety training



## RANDOM SAMPLING OF CONSTRUCTION MATERIALS

01

### Significance

Sampling and testing are two of the most important functions in quality control (QC). Data from the tests are the tools with which the quality of product is controlled. For this reason, great care must be used in following standardized sampling and testing procedures.

02

In controlling operations, it is necessary to obtain numerous samples at various points along the production line. Unless precautions are taken, sampling can occur in patterns that can create a bias to the data gathered. Sampling at the same time, say noon, each day may jeopardize the effectiveness of any quality program. This might occur, for example, because a material producer does certain operations, such as cleaning screens at an aggregate plant, late in the morning each day. To obtain a representative sample, a reliable system of random sampling must be employed.

### Scope

The procedure presented here eliminates bias in sampling materials. Randomly selecting a set of numbers from a table or calculator will eliminate the possibility for bias. Random numbers are used to identify sampling times, locations, or points within a lot or subplot. This method does not cover how to sample, but rather how to determine sampling times, locations, or points.

03

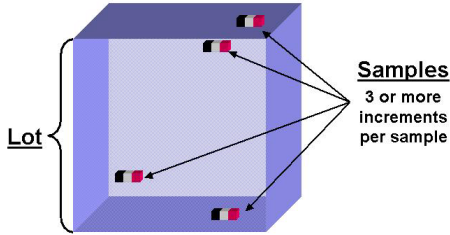
04

### Sampling Concepts

A lot is the quantity of material evaluated by QC procedures. A lot is a preselected quantity that may represent hours of production, a quantity or number of loads of material, or an interval of time. A lot may be comprised of several portions that are called sublots or units. The number of sublots comprising a lot will be determined by the agency's specifications.

### Straight Random Sampling

One or more sample locations may be selected, using the entire lot as a single unit



05

### Straight Random Sampling vs. Stratified

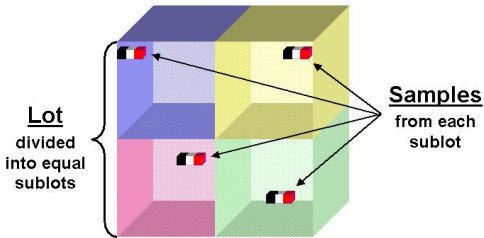
**Random Sampling:** Straight random sampling considers an entire lot as a single unit and determines each sample location based on the entire lot size. Stratified random sampling divides the lot into a specified number of sublots or units and then determines each sample location within a distinct subplot. Both methods result in random distribution of samples to be tested for compliance with the agency’s specification.

Agencies stipulate when to use straight random sampling or stratified random sampling. AASHTO R 90, Sampling Aggregate Products, for example, specifies a straight random sampling procedure.

06

### Stratified Random Sampling

The lot is divided into two or more equal sublots. Samples are taken from each subplot



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### Picking Random Numbers from a Table

Table 1 contains pairs of numbers. The first number is the “pick” number and the second is the Random Number, “RN”. The table was generated with a spreadsheet and the cells (boxes at the intersection of rows and columns) containing the RNs actually contain the “random number function.” Every time the spreadsheet is opened or changed, all the RNs change.

1. Select a Pick number in a random method. The first two or last two digits in the next automobile license plate you see would be one way to select. Another would be to start a digital stop watch and stop it several seconds later, using the decimal part of the seconds as your Pick number.
2. Find the RN matching the Pick number.

### Picking Random Numbers with a Calculator

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Many calculators have a built-in random number function. To obtain a random number, key in the code or push the button(s) the calculator’s instructions call for. The display will show a number between 0.000 and 1.000 and this will be your random number.

**TABLE 1**  
**Random Numbers**

Pick	RN	Pick	RN	Pick	RN	Pick	RN	Pick	RN
01	0.998	21	0.758	41	0.398	61	0.895	81	0.222
02	0.656	22	0.552	42	0.603	62	0.442	82	0.390
03	0.539	23	0.702	43	0.150	63	0.821	83	0.468
04	0.458	24	0.217	44	0.001	64	0.187	84	0.335
05	0.407	25	0.000	45	0.521	65	0.260	85	0.727
06	0.062	26	0.781	46	0.462	66	0.815	86	0.708
07	0.370	27	0.317	47	0.553	67	0.154	87	0.161
08	0.410	28	0.896	48	0.591	68	0.007	88	0.893
09	0.923	29	0.848	49	0.797	69	0.759	89	0.255
10	0.499	30	0.045	50	0.638	70	0.925	90	0.604
11	0.392	31	0.692	51	0.006	71	0.131	91	0.880
12	0.271	32	0.530	52	0.526	72	0.702	92	0.656
13	0.816	33	0.796	53	0.147	73	0.146	93	0.711
14	0.969	34	0.100	54	0.042	74	0.355	94	0.377
15	0.188	35	0.902	55	0.609	75	0.292	95	0.287
16	0.185	36	0.674	56	0.579	76	0.854	96	0.461
17	0.809	37	0.509	57	0.887	77	0.240	97	0.703
18	0.105	38	0.013	58	0.495	78	0.851	98	0.866
19	0.715	39	0.497	59	0.039	79	0.678	99	0.616
20	0.380	40	0.587	60	0.812	80	0.122	00	0.759

**Examples of Straight Random Sampling Procedures Using Random Numbers**

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**Sampling from a Belt or Flowing Stream:**

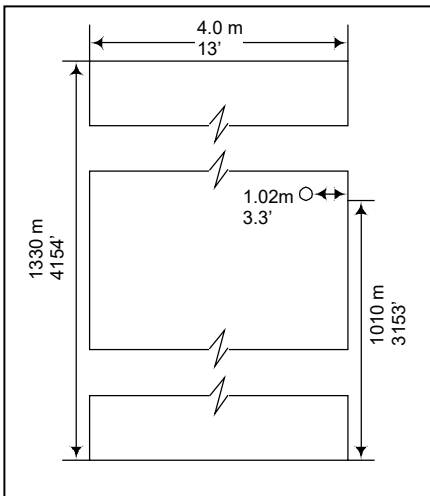
Agencies specify the frequency of sampling in terms of time, volumes, or masses. The specification might call for one sample from every 1,000,000 kg(1000 t) or 1100 Tons(T) of aggregate. If the random number was 0.317, the sample would be taken at  $(0.317)(1,000,000 \text{ kg}) = 317,000 \text{ kg}$  (317 t). Or  $(.317) (1100 \text{ T}) = 349 \text{ T}$ .

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One sample per day might also be specified. If the day were 9 hours long and the random number 0.199, the sample would be taken at  $(0.199) (9 \text{ hrs}) = 1.79 \text{ hr} = 1 \text{ hr}, 48 \text{ minutes}$  into the day.

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**Sampling from Transport Units:** Based on the agency’s specifications – in terms of time, volume, or mass – determine the number of transport units that comprise a lot. Multiply the selected random number(s) by the number of units to determine which unit(s) will be sampled.



Sampling from a roadway

For example, if 20 transport units comprise a lot and one sample is needed, pick one RN. If the RN were 0.773, then the sample would be taken from the  $(0.773)(20) = 15.46$ , or 16th transport unit.

### Sampling from a Roadway with Previously

**Placed Material:** The agency's specified frequency of sampling – in time, volume, or mass – can be translated into a location on a job. For example, if a sample is to be taken every  $800 \text{ m}^3$  ( $1000 \text{ yd}^3$ ) and material is being placed 0.15 m (0.50 ft) thick and 4.0 m (13 ft) wide, then the lot is 1330 m (4154 ft) long. You would select two RNs in this case. To convert  $\text{yd}^3$  to  $\text{ft}^3$  multiply by 27.

The first RN would be multiplied by the length to determine where the sample would be taken along the project. The second would be multiplied by the width to determine where, widthwise, the sample would be taken. For example, a first RN of 0.759 would specify that the sample would be taken at  $(0.759)(1330 \text{ m})$  or  $(4154 \text{ ft}) = 1010 \text{ m}$  or 3153 ft from the beginning. A second RN of 0.255 would specify that the sample would be taken at  $(0.255)(4.0 \text{ m})$  or  $(13 \text{ ft}) = 1.02 \text{ m}$  or 3.3 ft from the right edge of the material. To avoid problems associated with taking samples too close to the edge, no sample is taken closer than 0.3 m (1 ft) to the edge. If the RN specifies a location closer than 0.3 m (1 ft), then 0.3 m (1 ft) is added to or subtracted from the distance calculated.

**Sampling from a Stockpile:** AASHTO R 90 recommends against sampling from stockpiles. However, some agencies use random procedures in determining sampling locations from a stockpile. Bear in mind that stockpiles are prone to segregation and that a sample obtained from a stockpile may not be representative. Refer to AASHTO R 90 for guidance on how to sample from a stockpile.

**In-Place Density Testing:** Agency specifications will indicate the frequency of tests. For example, one test per  $500 \text{ m}^3$  ( $650 \text{ yd}^3$ ) might be required. If

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the material is being placed 0.15 m (0.50 ft) thick and 10.0 m (33 ft) wide, then the lot is 333 m (1090 ft) long. You would select two RNs in this case.

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The first RN would be multiplied by the length to determine where the sample would be taken along the project. The second would be multiplied by the width to determine where, widthwise, the sample would be taken. For example, a first RN of 0.387 would specify that the sample would be taken at  $(0.387)(333 \text{ m})$  or  $(1090 \text{ ft}) = 129 \text{ m}$  or  $(422 \text{ ft})$  from the beginning. A second RN of 0.558 would specify that the sample would be taken at  $(0.588)(10.0 \text{ m})$  or  $(33 \text{ ft}) = 5.88 \text{ m}$  or  $(19 \text{ ft})$  from the right edge of the material. To avoid problems associated with taking samples too close to the edge, no sample is taken closer than 0.3 m (1 ft) to the edge. If the RN specifies a location closer than 0.3 m (1 ft), then 0.3 m (1 ft) is added to or subtracted from the distance calculated.



### BASICS OF COMPACTION AND DENSITY CONTROL

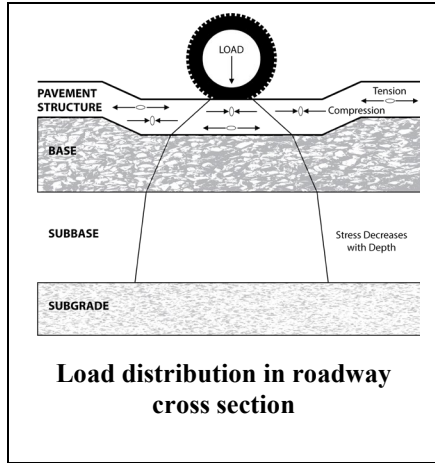
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#### Roadways

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Roadways are constructed in layers. The first (bottom) layer is the subgrade, or naturally present material. Next is the subbase, imported material usually having better structural, drainage, and other properties. Above the subbase is the base, material of even better structural quality than the subbase. Finally, there is the pavement consisting of either asphalt mixture or portland cement concrete (PCC). In this layered system, structural or load bearing properties improve as we move up from subgrade to pavement. The result is an economical roadway structure that supports traffic without undergoing excessive surface deflection and/or long-term settlement.

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Variations to this layering can occur such as roadways constructed on high quality subgrade so that the subbase layer can be eliminated. Also, there may be “embankment,” material between the naturally occurring subgrade and the subbase or base that is added in “fill” sections of the roadway where the finished road is substantially above original grade.

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Stability and durability of roadways depend on the final density of each component. Low-density material will lead to excessive surface deflection under load and/or long-term settlement. However, compacting material to densities higher than necessary costs both time and money.

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The quality of roadways also depends on the quality of the pavement. In asphalt mixture roadways, the density of the asphalt mixture plays a significant role in the overall ability to support load and provide long term service. Asphalt mixture pavement specifications include requirements for density and percent voids.

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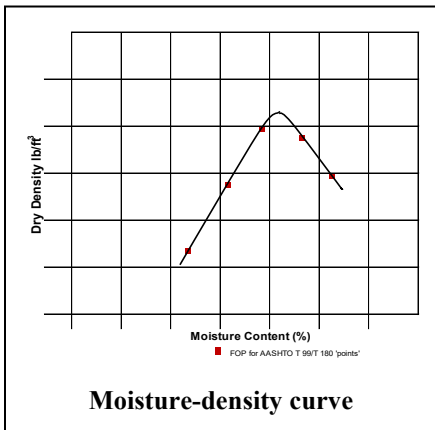




Sheepsfoot roller



Proctor molds and hammer



Moisture-density curve

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For these reasons, a basic understanding of compaction theory and a thorough knowledge of testing methods is necessary. Compaction equipment and techniques depend on the type of material. Cohesive soils, such as clay, and cohesionless soils, such as gravel, require different compaction methods, and different equipment is often used on asphalt mixtures than soils.

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### Fine-Grained Soils

For fine-grained soils that contain a significant amount of cohesion and little or no internal friction, density depends on compactive effort and moisture content. With these soils, moisture-density relations are key, and two similar test methods are used to determine the relationship between soil moisture and density.

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- AASHTO T 99
- AASHTO T 180

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In both methods, samples of soil are prepared at several moisture contents and compacted into molds of specified sizes using manual or mechanical rammers delivering a specified quantity of compactive energy. Knowing the wet masses of the compacted samples and the volume of the molds, wet densities can be determined. Moisture contents of the compacted samples are determined and used to obtain dry density values for the same samples. Maximum dry density and optimum moisture content for the soil are determined by plotting the relationship between dry density and moisture content on a moisture-density curve. The peak of the best fit curve for the data points indicates the maximum dry density and optimum moisture.

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Construction specifications generally require that the soil be compacted to some percentage of maximum dry density while being maintained at a moisture content close to the optimum. These specified values will be based on AASHTO T 99 or AASHTO T 180, depending on the agency. In the field, in-place dry density and moisture content of the material will be determined using a nuclear moisture-density gauge. The in-place values will be



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compared to the specifications to determine conformance with the project requirements.

**Correction for Oversize Material**

The FOP for AASHTO T 99/T 180 is conducted on materials passing either 4.75 mm (No. 4) or 19.0 mm (3/4 in.) sieve, depending on the method. If the material includes larger particles, corrections to the maximum dry density determination is required. The corrections are determined using the *Correction of Maximum Dry Density and Optimum Moisture for Oversized Particles Annex A* in the FOP for AASHTO T 99/T 180.

The corrected density is a weighted average of the density of the material passing the specified sieve and the material retained on the sieve. The density of the passing material is determined according to the FOP for AASHTO T 99/T 180. The density of the retained material is based on its bulk specific gravity ( $G_{sb}$ ), may be assumed according to the FOP for AASHTO T 99/T 180 or determined according to the FOP for AASHTO T 85.

**Granular Soil and Soil/Aggregates**

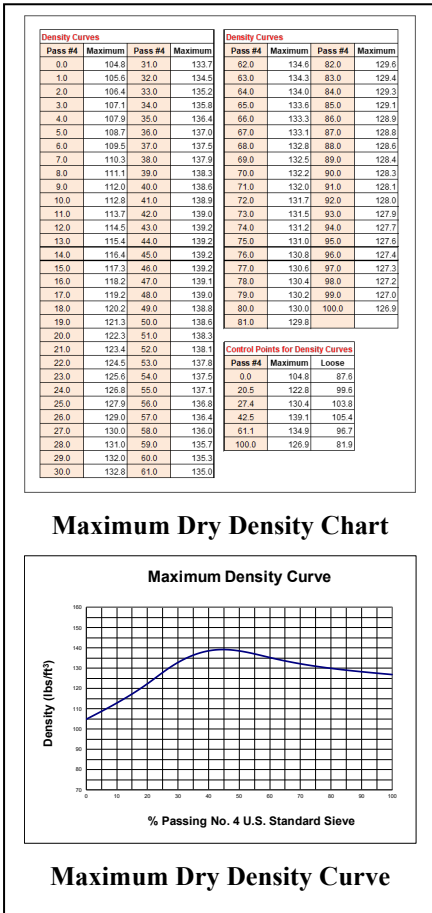
For coarse-grained granular soil and soil/aggregate having little or no cohesion, compactive effort is the primary concern. Although moisture is not as significant an issue, because materials are free-draining and do not retain water, it does assist in compaction by acting as a lubricant.

Granular, free-draining materials are tested by procedures that combine compaction and vibration. Various transportation agencies have developed specialized tests that are a hybrid of moisture-density test procedures and relative density determinations, including the following:

- AKDOT&PF's ATM 212
- ITD's IT 74
- WSDOT's T 606
- WFLHD's Humphres

These Agencies worked together to develop WAQTC TM 15, Laboratory Maximum Dry Density of Granular Soils and Soil/Aggregates and

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WAQTC TM 17, Determination of Theoretical Maximum Dry Density of Granular Soils and Soil/Aggregates for Use as a Density Standard.

22 In WAQTC TM 15, material is compacted in a mold with a combination of applied loads and vibration. A laboratory maximum dry density of the coarse and fine material is determined. Using the apparent specific gravity ( $G_{sa}$ ) and the  
23 laboratory maximum dry density, a maximum dry  
24 density curve and chart, representing the  
25 relationship between the maximum dry density and material passing the 4.75 mm (No. 4), are developed for the material.

**Backfill and Structural Backfill**

26 Backfill is replacing material that has been removed for excavation of pipe, utilities, structures, etc.  
27 Many of these placements are in the Roadway or near the Roadway Section.

28 Backfill material is placed in lifts (layers) and compacted with mechanical rollers and tamping devices, each layer must be tested to determine appropriate compaction and adherence to specifications.

29 Proper backfill placement and compaction are important to the stability of a structure, especially those without deep foundations such as pipes, retaining walls and bridges on spread footings.

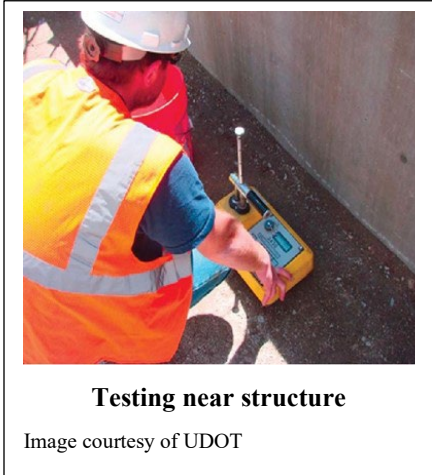
30 An uncompacted or unevenly compacted backfilled trench in the roadway, such as a pipe crossing, can lead to settlement of the roadway section.

31 Testing for in-place density and moisture content according to the FOP for AASHTO T 310 in trenches and near large objects requires extra steps to correct for ‘trench wall effect.’ Moisture present in the walls can be misread by the detector in the gauge. In locations closer than 600 mm (24 in.) to a vertical projection, it is necessary to perform a trench offset correction to adjust the gauge readings.



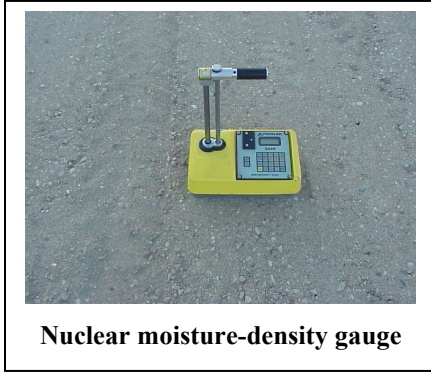
**Backfill compaction**

Image courtesy of UDOT



**Testing near structure**

Image courtesy of UDOT



Nuclear moisture-density gauge

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**In-Place Density**

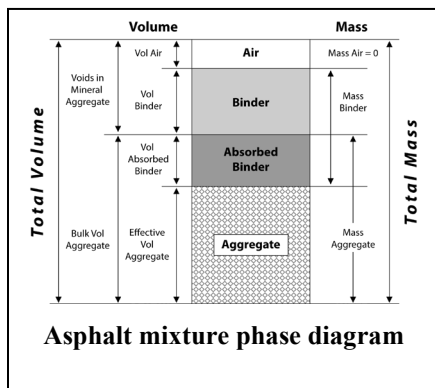
Construction specifications require in-place density to meet a minimum percent of maximum dry density.

For fine-grained soils, the in-place dry density and moisture content of the material is determined according to the FOP for AASHTO T 310 using a nuclear moisture-density gauge. The in-place dry density is compared to the maximum dry density and optimum moisture content obtained according to the FOP for AASHTO T 99/T 180, to determine percent compaction.

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For coarse grained granular soil and soil /aggregate, the in-place dry density is determined according to the FOP for AASHTO T 310. The percent of material passing the 4.75 mm (No. 4) sieve is determined on a sample from the density test site according to WAQTC TM 17. Using the percent passing, the maximum dry density is obtained from the maximum dry density chart. The in-place density percent compaction is compared to this maximum dry density to determine percent compaction.

$$\% \text{ Compaction} = \frac{\text{In place Density}}{\text{Maximum Density}} \times 100$$



Asphalt mixture phase diagram

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**Asphalt Mixture Pavement**

For asphalt mixtures, density depends on compactive effort as well as the mix design. The gradation and particle shape of the aggregate, the grade of asphalt binder, and the interaction of these have major influences on density and percent voids. The level of compactive effort and the equipment used depend on the mix design properties, environmental conditions, and lift thickness.

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Construction specifications will require a certain percentage of maximum voidless density, while maintaining voids within a certain range. A specification of 92 to 96 percent of maximum density and a corresponding void content between 8 and 4 percent is typical. Under-compaction results in low density and high void content. An under-compacted pavement will have low strength, reduced durability, high deformation, and high

38 permeability leading to problems such as rutting,  
39 raveling, and freeze-thaw damage. Over-  
compaction results in high density and low void  
content which may cause bleeding, rutting,  
cracking, or premature failure.

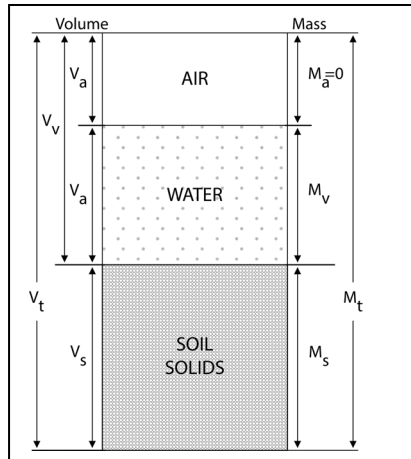
40 In the field, the in-place density of the compacted  
asphalt mixture is determined using core samples  
and/or according to the FOP for AASHTO T 355  
using a calibrated nuclear density gauge. The in-  
place density is compared to the theoretical  
maximum density obtained from the FOP for  
AASHTO T 209 to determine the percent air voids  
and percent compaction to determine conformance  
with the project specifications.

**Summary**

42 Proper compaction of soil, aggregate, and asphalt  
mixtures is necessary for high-quality roadways.  
Understanding and proper performance of  
standardized density tests are paramount in  
obtaining that compaction. The Embankment &  
Base and In-Place Density technicians must obtain  
samples and perform tests according to proper  
procedures in order to assure the quality of the  
finished roadway.

**TOTAL EVAPORABLE MOISTURE CONTENT OF AGGREGATE BY DRYING  
FOP FOR AASHTO T 255**

**LABORATORY DETERMINATION OF MOISTURE CONTENT OF SOILS  
FOP FOR AASHTO T 265**



**Phase diagram**



**Apparatus**



**Containers with lids for drying soils**

**Significance**

The amount of water contained in many materials influences design and construction practices. Road bases are difficult to compact if they are too dry or too wet. If too dry, water must be added, and the amount to be added depends on how much is already present.

**Scope**

This procedure covers the determination of moisture content of aggregate and soil in accordance with AASHTO T 255-25 and AASHTO T 265-22. It may also be used for other construction materials.

**Overview**

Moisture content is determined by comparing the wet mass of a sample and the mass of the sample after drying to constant mass. The term constant mass is used to define when a sample is dry.

*Constant mass* – the state at which a mass does not change more than a given percent, after additional drying for a defined time interval, at a required temperature.

**Apparatus**

- Balance or scale: capacity sufficient for the principal sample mass, readable to 0.1 percent of sample mass or 0.1 g., and meeting the requirements of AASHTO M 231
- Container: clean, dry, and capable of being sealed
- Suitable drying container
  - For soils: container requires close-fitting lid
  - For aggregate: container lid is optional



**Forced Air Oven**

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**Infrared Oven**

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- Microwave safe container with ventilated lid (for drying aggregate only)
- Heat source, thermostatically controlled, capable of maintaining  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ).
- Forced draft oven (preferred)
- Ventilated oven
- Convection oven
- Heat source, uncontrolled, for use when allowed by the agency, which will not alter the material being dried, and when close control of the temperature is not required:
  - Infrared heater/heat lamp, hot plate, fry pan, or any other device/method allowed by the agency
  - Microwave oven (900 watts minimum)
- Utensils such as spoons
- Hot pads or gloves

**Sample Preparation**

Obtain the sample in accordance with the FOP for AASHTO R 90 in its existing condition. If necessary, reduce the sample to moisture content sample size according to the FOP for AASHTO R 76.

For aggregate, the moisture content sample size is based on Table 1 or other information that may be specified by the agency.

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**TABLE 1**  
**Sample Sizes for Moisture Content of Aggregate**

<b>Nominal Maximum Size* mm (in.)</b>	<b>Minimum Sample Mass g (lb)</b>
150 (6)	50,000 (110)
100 (4)	25,000 (55)
90 (3 1/2)	16,000 (35)
75 (3)	13,000 (29)
63 (2 1/2)	10,000 (22)
50 (2)	8000 (18)
37.5 (1 1/2)	6000 (13)
25.0 (1)	4000 (9)
19.0 (3/4)	3000 (7)
12.5 (1/2)	2000 (4)
9.5 (3/8)	1500 (3.3)
4.75 (No. 4)	500 (1.1)

\* One sieve larger than the first sieve to retain more than 10 percent of the material using an agency specified set of sieves based on cumulative percent retained. Where large gaps in specification sieves exist, intermediate sieve(s) may be inserted to determine nominal maximum.

For soil, the moisture content sample size is based on Table 2 or other information that may be supplied by the agency.

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**TABLE 2**  
**Sample Sizes for Moisture Content of Soil**

<b>Maximum Particle Size mm (in.)</b>	<b>Minimum Sample Mass g</b>
50 (2)	1000
25.0 (1)	500
12.5 (1/2)	300
4.75 (No. 4)	100
0.425 (No. 40)	10

Immediately seal or cover moisture content samples to prevent any change in moisture content or follow the steps in "Procedure."

**Procedure**

Determine and record the sample masses as follows:

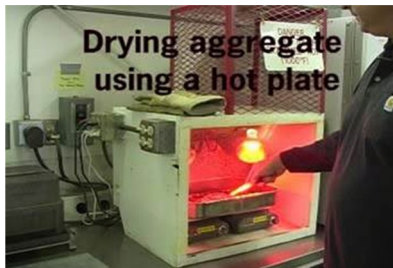
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- For aggregate, determine and record all masses to the nearest 0.1 percent of the sample mass or to the nearest 0.1 g.
- For soil, determine and record all masses to the nearest 0.1 g.

When determining the mass of hot samples or containers or both, place and tare a buffer between the sample container and the balance. This will eliminate damage to or interference with the operation of the balance or scale.

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1. Determine and record the mass of the container.
  - a. For aggregate: the lid is optional unless drying with a microwave then a ventilated lid is required.
  - b. For soils: the container includes the mass of the close-fitting lid.
2. Place the wet sample in the container.
3. Determine and record the total mass of the wet sample and container with lid, when used.
  - a. For oven(s), hot plates, infrared heaters, etc.: Spread the sample in the container.
  - b. For microwave oven: Heap sample in the container; cover with ventilated lid.



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4. Subtract the container mass determined in Step 1 from the mass of the container and sample determined in Step 3 and record the wet mass of the sample ( $M_w$ ).
5. Remove the lid, if used, and place the sample in one of the following drying apparatuses:
  - a. For aggregate –
    - i. Controlled heat source (oven): at  $110 \pm 5^\circ\text{C}$  ( $230 \pm 9^\circ\text{F}$ ).
    - ii. Uncontrolled heat source (Hot plate, infrared heater, or other heat source as

**Hotplate**

allowed by the agency): Stir frequently to avoid localized overheating.

- b. For soil – controlled heat source (oven): at  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ).

*Note 1:* Soils containing gypsum or significant amounts of organic material require special drying. For reliable moisture contents, dry these soils at  $60^{\circ}\text{C}$  ( $140^{\circ}\text{F}$ ). For more information see AASHTO T 265, Note 2.

6. Dry until sample appears moisture free.

22 7. Determine mass of sample and container with lid, when used.

8. Subtract the container mass determined in Step 1 from the mass of the container and sample determined in Step 7 and record the mass of the sample.

23 9. Return sample and container to the heat source for the additional time interval.

a. Drying intervals for aggregate –

i. Controlled heat source (oven):  
30 minutes

ii. Uncontrolled heat source (Hot plate, infrared heater, or other heat source as allowed by the agency): 10 minutes

iii. Uncontrolled heat source (Microwave oven): 2 minutes

**Caution:** Some minerals in the sample may cause the aggregate to overheat, crack, and explode; altering the aggregate gradation.

**Table 3  
Methods of Drying**

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<b>Aggregate</b>		
<b>Heat Source</b>	<b>Specific Instructions</b>	<b>Drying intervals to achieve constant mass (minutes)</b>
<b>Controlled:</b>		
Forced draft (preferred), ventilated, or convection oven	110 ±5°C (230 ±9°F)	30
<b>Uncontrolled:</b>		
Hot plate, infrared heater, or any other device/method allowed by the agency	Stir frequently	10
Microwave	Heap sample and cover with ventilated lid	2

b. Drying interval for soil – controlled heat source (oven): 1 hour

**Table 4  
Method of Drying**

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<b>Soil</b>		
<b>Heat Source</b>	<b>Specific Instructions</b>	<b>Drying interval to achieve constant mass</b>
<b>Controlled:</b>		
Forced draft (preferred), ventilated, or convection oven	110 ±5°C (230 ±9°F)	1 hour

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10. Determine mass of sample and container with lid, when used.

11. Subtract the container mass determined in Step 1 from the mass of the container and sample determined in Step 10 and record the mass of the sample.

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12. Determine percent change by subtracting the new mass determination ( $M_n$ ) from the previous mass determination ( $M_p$ ), dividing by

- 28 the previous mass determination ( $M_p$ ), and multiplying by 100.
13. Continue drying, performing steps 9 through 12, until there is less than a 0.10 percent change after additional drying time.
14. Constant mass has been achieved; sample is defined as dry.
15. Allow the sample and container with lid, when used, to cool. Determine and record the total mass.
16. Subtract the mass of the container determined in Step 1 from the mass of the container and sample determined in Step 15 and record the dry mass of the sample ( $M_D$ ).
- 29 17. Determine and record percent moisture ( $w$ ) by subtracting the final dry mass determination ( $M_D$ ) from the initial wet mass determination ( $M_W$ ), dividing by the final dry mass
- 30 determination ( $M_D$ ), and multiplying by 100.

## Calculation

### Constant Mass

Calculate constant mass using the following formula:

$$\% \text{ Change} = \frac{M_p - M_n}{M_p} \times 100$$

Where:

$M_p$  = previous mass measurement

$M_n$  = new mass measurement

**Constant Mass Example:**

Mass of container:	1232.1 g	30
Mass of the container and sample after first drying cycle:	2637.2 g	
Mass, $M_p$ , of possibly dry sample:	$2637.2 \text{ g} - 1232.1 \text{ g} = 1405.1 \text{ g}$	
Mass of container and sample after second drying cycle:	2634.1 g	
Mass, $M_n$ , of sample:	$2634.1 \text{ g} - 1232.1 \text{ g} = 1402.0 \text{ g}$	

$$\% \text{ Change} = \frac{1405.1 \text{ g} - 1402.0 \text{ g}}{1405.1 \text{ g}} \times 100 = 0.22\%$$

*0.22 percent is not less than 0.10 percent, so continue drying*

Mass of container and sample after third drying cycle: 2633.0 g  
 Mass,  $M_n$ , of sample:  $2633.0 \text{ g} - 1232.1 \text{ g} = 1400.9 \text{ g}$

$$\% \text{ Change} = \frac{1402.0 \text{ g} - 1400.9 \text{ g}}{1402.0 \text{ g}} \times 100 = 0.08\% \quad 31$$

*0.08 percent is less than 0.10 percent, so constant mass has been reached.*

**Moisture Content Aggregate and Soils:**

Calculate the moisture content, as a percent, using the following formula:

27

$$w = \frac{M_W - M_D}{M_D} \times 100$$

where:

- w = moisture content, percent
- $M_W$  = wet mass
- $M_D$  = dry mass

Example:

Mass of container:	1232.1 g	32
Mass of container and wet sample:	2764.7 g	
Mass, $M_W$ , of wet sample:	$2764.7 \text{ g} - 1232.1 \text{ g} = 1532.6 \text{ g}$	
Mass of container and dry sample ( <b>COOLED</b> ):	2633.5 g	
Mass, $M_D$ , of dry sample:	$2633.5 \text{ g} - 1232.1 \text{ g} = 1401.4 \text{ g}$	

$$w = \frac{1532.6 \text{ g} - 1401.4 \text{ g}}{1401.4 \text{ g}} \times 100 = \frac{131.2 \text{ g}}{1401.4 \text{ g}} \times 100 = 9.36\% \text{ report } 9.4\%$$

33

### Report

- On forms approved by the agency
- Sample ID
- $M_W$ , wet mass
- $M_D$ , dry mass
- $w$ , moisture content to the nearest 0.1 percent

34

### Tips!

- Let the sample cool before determining final dry mass.
- Divide by  $M_D$ , not  $M_W$ .



**REVIEW QUESTIONS**

1. What extra care should be taken when using a microwave to dry aggregates?
  
  
  
  
  
  
  
  
  
  
2. What is the maximum temperature that a sample should be allowed to attain for each of the various types of ovens?
  
  
  
  
  
  
  
  
  
  
3. How is “constant mass” defined according to this FOP:

For Aggregate?

For Soil?



**PERFORMANCE EXAM CHECKLIST**

**TOTAL EVAPORABLE MOISTURE CONTENT OF AGGREGATE BY DRYING  
FOP FOR AASHTO T 255**

**LABORATORY DETERMINATION OF MOISTURE CONTENT OF SOILS  
FOP FOR AASHTO T 265**

Participant Name \_\_\_\_\_ Exam Date \_\_\_\_\_

Record the symbols “P” for passing or “F” for failing on each step of the checklist.

<b>Procedure Element</b>	<b>Trial 1</b>	<b>Trial 2</b>
1. Representative sample of appropriate mass obtained?	_____	_____
2. Sample protected from moisture change?	_____	_____
3. Mass of container determined to 0.1 g?	_____	_____
a. For aggregate: the lid is optional unless drying with a microwave then a ventilated lid is required.		
b. For soils: the container includes the mass of the close-fitting lid.		
4. Sample placed in container, with lid if used, and mass determined to 0.1 g?	_____	_____
5. Wet sample mass recorded to 0.1 g?	_____	_____
6. Sample mass conforms to the required mass?	_____	_____
7. Sample dried by a suitable heat source?	_____	_____
a. Describe suitable heat sources for aggregate?		
b. Describe suitable heat sources for soils?		
8. If aggregate heated by means other than a controlled oven, is sample stirred to avoid localized overheating?	_____	_____
9. For microwave, aggregate heaped and covered with a ventilated lid?	_____	_____
10. For aggregate, heated for the additional, specified time?	_____	_____
a. Forced draft, ventilated, convection ovens – 30 minutes		
b. Microwave – 2 minutes		
c. Other – 10 minutes		
11. For soil, heated for at least 1 hour additional drying time using a controlled heat source?	_____	_____
12. Mass of sample and container, with lid if used, determined?	_____	_____
13. Constant mass checked at drying intervals. Final interval showing less than 0.10 percent loss?	_____	_____

**OVER**

14. Sample cooled, dry mass and container, with lid if used, determined and recorded to the nearest 0.1 g? \_\_\_\_\_

15. Moisture content calculated correctly and recorded to the nearest 0.1 percent? \_\_\_\_\_

Comments: First attempt: Pass \_\_\_\_\_ Fail \_\_\_\_\_ Second attempt: Pass \_\_\_\_\_ Fail \_\_\_\_\_

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Examiner Signature \_\_\_\_\_ WAQTC #: \_\_\_\_\_

**MOISTURE-DENSITY RELATIONS OF SOILS:**

**USING A 2.5 KG (5.5 LB) RAMMER AND A 305 MM (12 IN.) DROP  
FOP FOR AASHTO T 99**

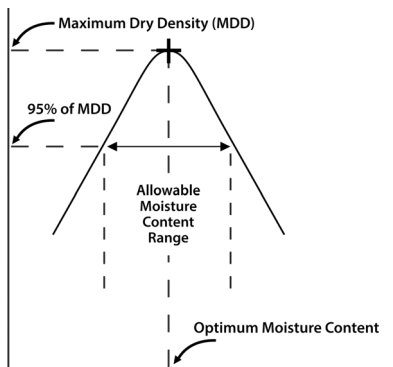
**USING A 4.54 KG (10 LB) RAMMER AND A 457 MM (18 IN.) DROP  
FOP FOR AASHTO T 180**



**Steel roller**



**Adding water**



**Moisture vs. dry density**

**Significance**

01  
02

The density or degree of compaction, of soil or soil-aggregate mixtures has a significant influence on the stability and durability of roadways. Low density subgrade, subbase, base, or embankment will lead to excessive deflection under load or long-term settlement in an amount higher than anticipated, or both. Obtaining proper density depends on two major factors: compactive effort and moisture content.

03  
04

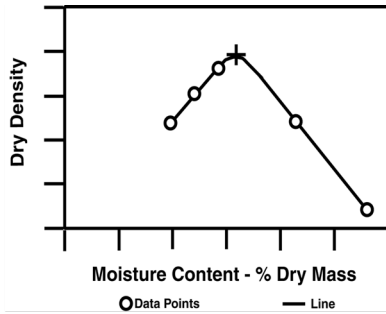
Compactive effort relates to the type and weight of compaction equipment, along with the thickness of the “lift” being compacted and the number of times each lift is passed over by the compaction equipment. Equipment includes static and vibratory rollers, smooth and sheepsfoot steel rollers, and pneumatic tire rollers of varied weights yielding many different compactive efforts.

05  
06

Density also depends upon moisture content. The moisture content corresponding to maximum dry density of the soil or soil-aggregate mixture under a given compactive effort is known as optimum water content. As the water content increases or decreases from this optimum value, the dry density decreases.

07  
08

Agency specifications commonly require that a certain percentage of maximum dry density be obtained while the moisture content of the soil or soil-aggregate mixture is held within certain limits. For example, a specification might call for 95 percent of maximum dry density with a moisture content of the optimum value  $\pm 2$  percent. For these reasons, it is critical to understand the various test methods and equipment used in determining the moisture-density relations of soil.



**Optimum water content**



**Molds and Rammer**

**Scope**

This procedure covers the determination of the moisture-density relations of soils and soil-aggregate mixtures in accordance with two similar test methods:

09

- AASHTO T 99-25: Methods A, B, C, and D
- AASHTO T 180-25: Methods A, B, C, and D

10

11

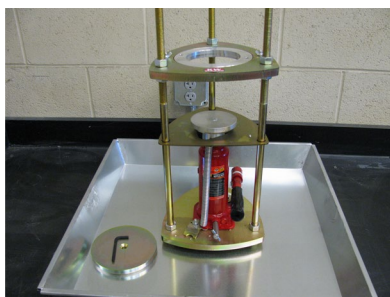
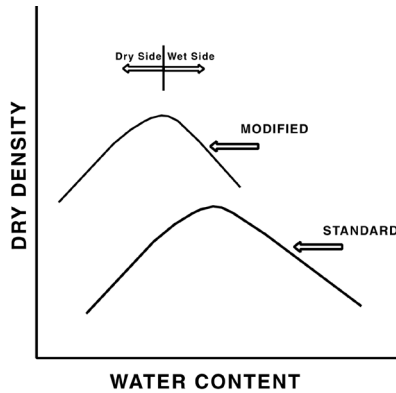
This test method applies to soil mixtures having 40 percent or less retained on the 4.75 mm (No. 4) sieve for methods A or B, or 30 percent or less on the 19 mm (¾ in.) sieve with methods C or D. The retained material is defined as oversize (coarse) material. If no minimum percentage is specified, 5 percent will be used. Samples containing oversize (coarse) material that meet the percent retained criteria should be corrected by using *Annex A, Correction of Maximum Dry Density and Optimum Moisture for Oversized Particles*. Samples of soil or soil-aggregate mixture are prepared at several moisture contents and compacted into molds of specified size, using manual or mechanical rammers that deliver a specified quantity of compactive energy. The moist masses of the compacted samples are divided by the volume of the mold to determine wet density values. Moisture contents of the compacted samples are determined and used to obtain the dry density values of the same samples. Maximum dry density and optimum moisture content for the soil or soil-aggregate mixture is determined by plotting the relationship between dry density and moisture content.

**Apparatus**

- Mold – Cylindrical mold made of metal with the dimensions shown in Table 1 or Table 2. If permitted by the agency, the mold may be of the “split” type, consisting of two half-round sections, which can be securely locked in place to form a cylinder. Determine the mold volume according to *Annex B, Standardization of the Mold*.
- Mold assembly – Mold, base plate, and a detachable collar.



**Mechanical Rammer**



**Sample extruder**

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- Rammer – Manually or mechanically operated rammers as detailed in Table 1 or Table 2. A manually operated rammer shall be equipped with a guide sleeve to control the path and height of drop. The guide sleeve shall have at least four vent holes no smaller than 9.5 mm (3/8 in.) in diameter, spaced approximately 90 degrees apart and approximately 19 mm (3/4 in.) from each end. A mechanically operated rammer will uniformly distribute blows over the sample and will be calibrated with several soil types, and shall be adjusted, if necessary, to give the same moisture-density results as with the manually operated rammer. For additional information concerning calibration, see AASHTO T 99 and T 180.
- Sample extruder – A jack, lever frame, or other device for extruding compacted specimens from the mold quickly and with little disturbance.
- Balance(s) or scale(s) of the capacity and sensitivity required for the procedure used by the agency.
  - A balance or scale with a capacity of 11.5 kg (25 lb) and a sensitivity of 1 g for obtaining the sample, meeting the requirements of AASHTO M 231, Class G 5.
  - A balance or scale with a capacity of 2 kg and a sensitivity of 0.1 g is used for moisture content determinations done under both procedures, meeting the requirements of AASHTO M 231, Class G 5.
- Drying oven – Controlled drying oven, capable of maintaining a temperature of  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ) for drying moisture content samples in accordance with the FOP for AASHTO T 255/T 265.
- Straightedge – A steel straightedge at least 250 mm (10 in.) long, with one beveled edge and at least one surface plane within 0.1 percent of its length, used for final trimming.



4.75 mm (No. 4) sieve - Straight edge

- Sieve(s) – 4.75 mm (No. 4) and/or 19.0 mm (3/4 in.), meeting the requirements of FOP for AASHTO T 27/T 11.
- Mixing tools – Miscellaneous tools such as a mixing pan, spoon, trowel, spatula, etc., or a suitable mechanical device, for mixing the sample with water.
- Containers with close-fitting lids to prevent gain or loss of moisture in the sample.

**Table 1**  
**Comparison of Apparatus, Sample, and Procedure - Metric**

	<b>T 99</b>	<b>T 180</b>
Mold Volume, m <sup>3</sup>	Methods A, C: 0.000943 ±0.000014	Methods A, C: 0.000943 ±0.000014
	Methods B, D: 0.002124 ±0.000025	Methods B, D: 0.002124 ±0.000025
Mold Diameter, mm	Methods A, C: 101.60 ±0.40	Methods A, C: 101.60 ±0.4
	Methods B, D: 152.40 ±0.70	Methods B, D: 152.40 ±0.70
Mold Height, mm	116.40 ±0.50	116.40 ±0.50
Detachable Collar Height, mm	50.80 ±0.64	50.80 ±0.64
Rammer Diameter, mm	50.80 ±0.25	50.80 ±0.25
Rammer Mass, kg	2.495 ±0.009	4.536 ±0.009
Rammer Drop, mm	305 ±2	457 ±2
Layers	3	5
Blows per Layer	Methods A, C: 25	Methods A, C: 25
	Methods B, D: 56	Methods B, D: 56
Material Size, mm	Methods A, B: 4.75 minus	Methods A, B: 4.75 minus
	Methods C, D: 19.0 minus	Methods C, D: 19.0 minus
Test Sample Size, kg	Method A: 3	Method B: 7
	Method C: 5 (1)	Method D: 11(1)
Energy, kN-m/m <sup>3</sup>	592	2,693

(1) This may not be a large enough sample depending on your nominal maximum size for moisture content samples.

**Table 2**  
**Comparison of Apparatus, Sample, and Procedure - English**

	<b>T 99</b>	<b>T 180</b>
Mold Volume, ft <sup>3</sup>	Methods A, C: 0.0333 ±0.0005	Methods A, C: 0.0333 ±0.0005
	Methods B, D: 0.07500 ±0.0009	Methods B, D: 0.07500 ±0.0009
Mold Diameter, in.	Methods A, C: 4.000 ±0.016	Methods A, C: 4.000 ±0.016
	Methods B, D: 6.000 ±0.026	Methods B, D: 6.000 ±0.026
Mold Height, in.	4.584 ±0.018	4.584 ±0.018
Detachable Collar Height, in.	2.000 ±0.025	2.000 ±0.025
Rammer Diameter, in.	2.000 ±0.025	2.000 ±0.025
Rammer Mass, lb	5.5 ±0.02	10 ±0.02
Rammer Drop, in.	12 ±0.06	18 ±0.06
Layers	3	5
Blows per Layer	Methods A, C: 25	Methods A, C: 25
	Methods B, D: 56	Methods B, D: 56
Material Size, in.	Methods A, B: No. 4 minus	Methods A, B: No.4 minus
	Methods C, D: 3/4 minus	Methods C, D: 3/4 minus
Test Sample Size, lb	Method A: 7 Method C: 12 <sub>(1)</sub>	Method B: 16 Method D: 25 <sub>(1)</sub>
Energy, lb-ft/ft <sup>3</sup>	12,375	56,250

(1) This may not be a large enough sample depending on your nominal maximum size for moisture content samples.

	Sieve	4.75 mm (No. 4)	19 mm (3/4")
Mold			
4"		A	C
6"		B	D

**Methods**

**Sample**

If the sample is damp, dry it until it becomes friable under a trowel. Drying may be in air or by use of a drying apparatus maintained at a temperature not exceeding 60°C (140°F). Thoroughly break up aggregations in a manner that avoids reducing the natural size of individual particles.

Obtain a representative test sample of the mass required by the agency by passing the material through the sieve required by the agency. See Table 1 or Table 2 for test sample mass and material size requirements.

In instances where the material is prone to degradation, i.e., granular material, a compaction sample with differing moisture content should be prepared for each point.

If the sample is plastic (clay types), it should stand for a minimum of 12 hours after the addition of water to allow the moisture to be absorbed. In this case, several samples at different moisture contents should be prepared, put in sealed containers, and tested the next day.



**Compacting**



**Typical mold**

22

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*Note 1:* Both T 99 and T 180 have four methods (A, B, C, D) that require different masses and employ different sieves.

**Procedure**

During compaction, rest the mold firmly on a dense, uniform, rigid, and stable foundation, or base. This base shall remain stationary during the compaction process.

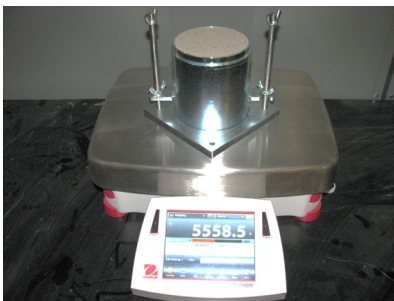
1. Determine the mass of the clean, dry mold. Include the base plate but exclude the extension collar. Record the mass to the nearest 1 g (0.005 lb).
2. Thoroughly mix the selected representative sample with sufficient water to dampen it to approximately 4 to 8 percentage points below optimum moisture content. For many materials, this condition can be identified by forming a cast by hand.
  - a. Prepare individual samples of plastic or degradable material, increasing moisture contents 1 to 2 percent for each point.
  - b. Allow samples of plastic soil to stand for at least 12 hrs.
3. Form a specimen by compacting the prepared soil in the mold assembly in approximately equal layers. For each layer:
  - a. Spread the loose material uniformly in the mold.
  - b. Lightly tamp the loose material with the manual rammer or other similar device, this establishes a firm surface.

*Note 2:* It is recommended to cover the remaining material with a non-absorbent sheet or damp cloth to minimize loss of moisture.

- c. Compact each layer with uniformly distributed blows from the rammer. See Table 1 for mold size, number of layers, number of blows, and rammer specification for the various test methods. Use the method specified by the agency.
- d. Trim down material that has not been compacted and remains adjacent to the walls



**Trimming**



**Mass of mold and wet soil**



**Extruding the material**

25

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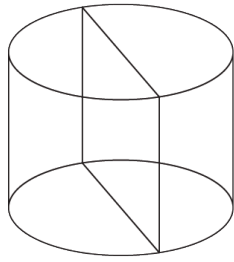
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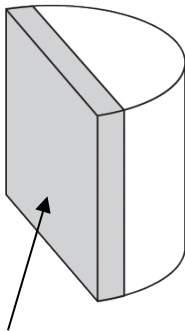
30

of the mold and extends above the compacted surface.

4. Remove the extension collar. Avoid shearing off the sample below the top of the mold. The material compacted in the mold should not be over 6 mm (1/4 in) above the top of the mold once the collar has been removed.
  5. Trim the compacted soil even with the top of the mold with the beveled edge of the straightedge.
  6. Clean soil from exterior of the mold and base plate.
  7. Determine the mass of the mold, base plate, and wet soil to the nearest 1 g (0.005 lb).
  8. Determine the wet mass of the sample by subtracting the mass in Step 1 from the mass in Step 7.
  9. Calculate the wet density ( $\rho_w$ ), in  $\text{kg/m}^3$  ( $\text{lb/ft}^3$ ), by dividing the wet mass by the measured volume ( $V_m$ ).
  10. Extrude the material from the mold. For soils and soil-aggregate mixtures, slice vertically through the center and remove one of the cut faces for a representative moisture content sample. For granular materials, a vertical face will not exist. Take a representative sample ensuring that all layers are represented. This sample must meet the sample size requirements of the test method used to determine moisture content.
- Note 3:* When developing a curve for free-draining soils such as uniform sands and gravels, where seepage occurs at the bottom of the mold and base plate, taking a representative moisture content from the mixing bowl may be preferred in order to determine the amount of moisture available for compaction.
11. Determine the moisture content ( $w$ ) of the sample in accordance with the FOP for AASHTO T 255 / T 265.
  12. If the material is degradable or plastic, return to Step 3 using a prepared individual sample. If not, continue with Steps 13 through 15.



**Extruded material**



**Representative moisture content sample**

31  
32  
33

13. Thoroughly break up the remaining portion of the molded specimen until it will again pass through the sieve, as judged by eye, and add to the remaining portion of the sample being tested.
14. Add sufficient water to increase the moisture content of the remaining soil by approximately 1 to 2 percentage points and repeat the above procedure.
15. Continue determinations until there is either a decrease or no change in the mass. There will be a minimum of three points on the dry side of the curve and two points on the wet side. For non-cohesive, drainable soils, one point on the wet side is sufficient.

**Calculations**

**Wet Density**

34

$$\rho_w = \frac{M_w}{V_m}$$

Where:

- $\rho_w$  = wet density, kg/m<sup>3</sup> (lb/ft<sup>3</sup>)
- $M_w$  = wet mass
- $V_m$  = volume of the mold, Annex B

**Dry Density**

35

$$\rho_d = \left( \frac{\rho_w}{w + 100} \right) \times 100 \quad \text{or} \quad \rho_d = \frac{\rho_w}{\left( \frac{w}{100} \right) + 1}$$

Where:

$$\begin{aligned} \rho_d &= \text{dry density, kg/m}^3 \text{ (lb/ft}^3\text{)} \\ w &= \text{moisture content, as a percentage} \end{aligned}$$

**Example for 4-inch mold, Methods A or C**

36

$$\begin{aligned} \text{Wet mass, } M_w &= 1.928 \text{ kg (4.250 lb)} \\ \text{Moisture content, } w &= 11.3\% \\ \text{Measured volume of the mold, } V_m &= 0.000946 \text{ m}^3 \text{ (0.0334 ft}^3\text{)} \end{aligned}$$

**Wet Density**

$$\rho_w = \frac{1.928 \text{ kg}}{0.000946 \text{ m}^3} = 2038 \text{ kg/m}^3 \quad \rho_w = \frac{4.250 \text{ lb}}{0.0334 \text{ ft}^3} = 127.2 \text{ lb/ft}^3$$

37

**Dry Density**

38

$$\rho_d = \left( \frac{2038 \text{ kg/m}^3}{11.3 + 100} \right) \times 100 = 1831 \text{ kg/m}^3 \quad \rho_d = \left( \frac{127.2 \text{ lb/ft}^3}{11.3 + 100} \right) \times 100 = 114.3 \text{ lb/ft}^3$$

Or

$$\rho_d = \left( \frac{2038 \text{ kg/m}^3}{\frac{11.3}{100} + 1} \right) = 1831 \text{ kg/m}^3 \quad \rho_d = \left( \frac{127.2 \text{ lb/ft}^3}{\frac{11.3}{100} + 1} \right) = 114.3 \text{ lb/ft}^3$$

**Moisture-Density Curve Development**

39

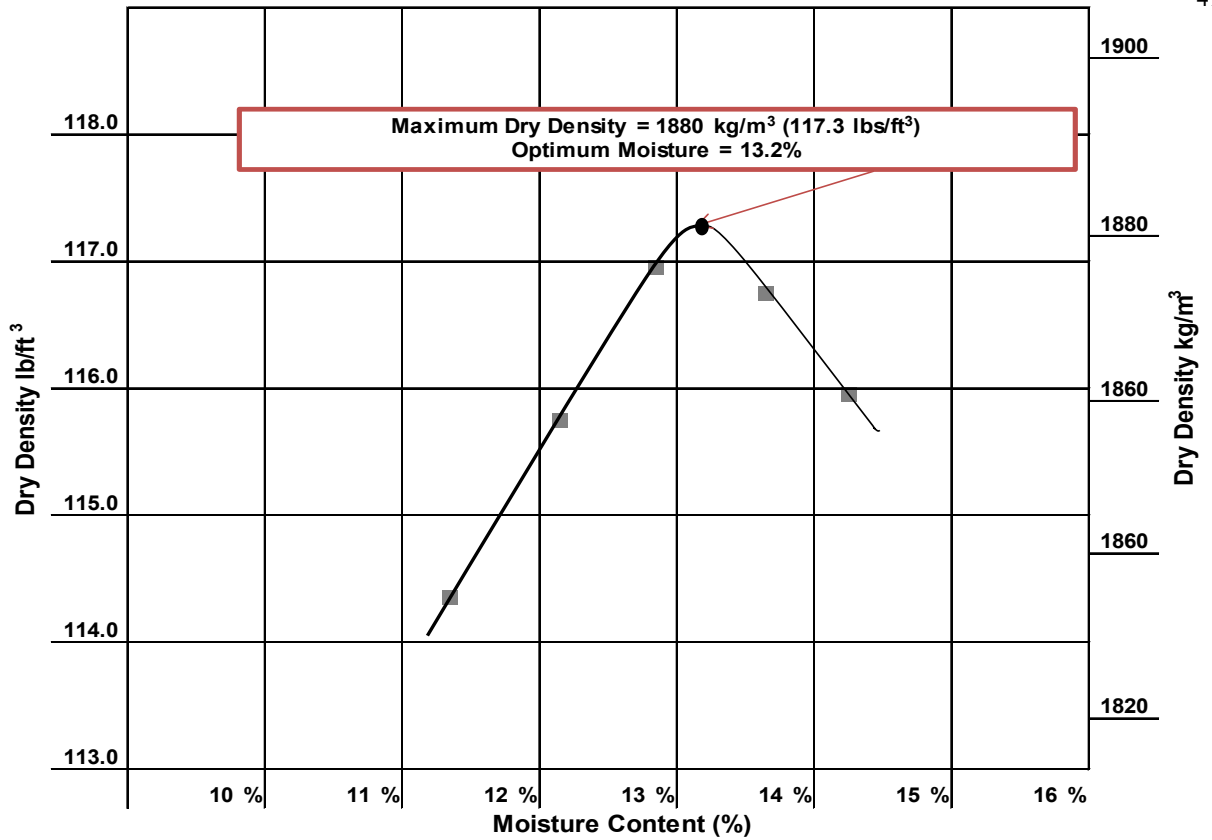
When dry density is plotted on the vertical axis versus moisture content on the horizontal axis and the points are connected with a smooth line, a moisture-density curve is developed. The coordinates of the peak of the curve are the maximum dry density, or just “maximum density,” and the “optimum moisture content” of the soil.

**Example**

Given the following dry density and corresponding moisture content values, develop a moisture-density relations curve and determine maximum dry density and optimum moisture content.

Dry Density,		Moisture Content, %
kg/m <sup>3</sup>	lb/ft <sup>3</sup>	
1831	114.3	11.3
1853	115.7	12.1
1873	116.9	12.8
1869	116.7	13.6
1857	115.9	14.2

40



In this case, the curve has its peak at:

Maximum dry density = 1880 kg/m<sup>3</sup> (117.3 lb/ft<sup>3</sup>)

Optimum moisture content = 13.2%

Note that both values are approximate since they are based on sketching the curve to fit the points.

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**Report**

- On forms approved by the agency
- Sample ID
- Maximum dry density to the nearest  $1 \text{ kg/m}^3$  ( $0.1 \text{ lb/ft}^3$ )
- Optimum moisture content to the nearest 0.1 percent

44

**Tips!**

- Ideally, obtain 3 dry points and 2 wet points. This produces a reliable moisture-density curve.
- Moisture-density curves are based on dry densities.
- If oversize material exists, corrections must be made.

---

## ANNEX A

### CORRECTION OF MAXIMUM DRY DENSITY AND OPTIMUM MOISTURE FOR OVERSIZED PARTICLES

2

(Mandatory Information)

This section corrects the maximum dry density and moisture content of the material retained on the 4.75 mm (No. 4) sieve, Methods A and B; or the material retained on the 19 mm (¾ in.) sieve, Methods C and D. The maximum dry density, corrected for oversized particles and total moisture content, are compared with the field-dry density and field moisture content.

3

This correction can be applied to the sample on which the maximum dry density is performed. A correction may not be practical for soils with only a small percentage of oversize material. The agency shall specify a minimum percentage below which the method is not needed. If not specified, this method applies when more than 5 percent by weight of oversize particles is present.

4

5

Bulk specific gravity ( $G_{sb}$ ) of the oversized particles is required to determine the corrected maximum dry density. Use the bulk specific gravity as determined using the FOP for AASHTO T 85 in the calculations. For construction activities, an agency established value or specific gravity of 2.600 may be used.

6

This correction can also be applied to the sample obtained from the field while performing in-place density.

#### Procedure

1. Use the sample from this procedure or a sample obtained according to the FOP for AASHTO T 310.
2. Sieve the sample on the 4.75 mm (No. 4) sieve for Methods A and B or the 19 mm (¾ in.) sieve, Methods C and D.
3. Determine the dry mass of the oversized and fine fractions ( $M_{DC}$  and  $M_{DF}$ ) by one of the following:
  - a. Dry the fractions, fine and oversized, in air or by use of a drying apparatus that is maintained at a temperature not exceeding 60°C (140°F).
  - b. Calculate the dry masses using the moisture samples.

7

To determine the dry mass of the fractions using moisture samples.

1. Determine the moist mass of both fractions, fine ( $M_{Mf}$ ) and oversized ( $M_{Mc}$ ):
2. Obtain moisture samples from the fine and oversized material.

8

3. Determine the moisture content of the fine particles ( $MC_f$ ) and oversized particles ( $MC_c$ ) of the material by FOP for AASHTO T 255/T 265 or agency approved method.
4. Calculate the dry mass of the oversize and fine particles.

$$M_D = \frac{M_m}{1 + MC}$$

9

Where:

$M_D$  = mass of dry material (fine or oversize particles).

$M_m$  = mass of moist material (fine or oversize particles).

MC = moisture content of respective fine or oversized, expressed as a decimal.

5. Calculate the percentage of the fine ( $P_f$ ) and oversized ( $P_c$ ) particles by dry weight of the total sample as follows: See Note 2.

$$P_f = \frac{100 \times M_{DF}}{M_{DF} + M_{DC}} \quad \frac{100 \times 15.4 \text{ lb}}{15.4 \text{ lbs} + 5.7 \text{ lb}} = 73\% \quad \frac{100 \times 6.985 \text{ kg}}{6.985 \text{ kg} + 2.585 \text{ kg}} = 73\%$$

10

And

$$P_c = \frac{100 \times M_{DC}}{M_{DF} + M_{DC}} \quad \frac{100 \times 5.7 \text{ lb}}{15.4 \text{ lbs} + 5.7 \text{ lb}} = 27\% \quad \frac{100 \times 2.585 \text{ kg}}{6.985 \text{ kg} + 2.585 \text{ kg}} = 27\%$$

11

Or for  $P_c$ :

$$P_c = 100 - P_f$$

Where:

$P_f$  = percent of fine particles, of sieve used, by weight

$P_c$  = percent of oversize particles, of sieve used, by weight

$M_{DF}$  = mass of dry fine particles

$M_{DC}$  = mass of dry oversize particles

## Optimum Moisture Correction Equation

12

1. Calculate the corrected moisture content as follows:

$$MC_T = \frac{(MC_F \times P_f) + (MC_c \times P_c)}{100}$$

$$MC_T = \frac{(10.6\% \times 73.0\%) + (2.1\% \times 27.0\%)}{100} = 8.3\%$$

13

Where:

$MC_T$  = corrected moisture content of combined fines and oversized particles, expressed as a % moisture

$MC_F$  = moisture content of fine particles, as a % moisture

$MC_C$  = moisture content of oversized particles, as a % moisture

**Note 1:** Moisture content of oversize material can be assumed to be two (2) percent for most construction applications.

**Note 2:** In some field applications agencies will allow the percentages of oversize and fine materials to be determined with the materials in the wet state.

## Density Correction Equation

2. Calculate the corrected dry density ( $\rho_d$ ) of the total sample (combined fine and oversized particles) as follows: 14

$$\rho_d = \frac{100\%}{\left[ \left( \frac{P_f}{\rho_f} \right) + \left( \frac{P_c}{k} \right) \right]} \quad 15$$

Where:

$\rho_d$  = corrected total dry density (combined fine and oversized particles)  
kg/m<sup>3</sup> (lb/ft<sup>3</sup>)

$\rho_f$  = dry density of the fine particles kg/m<sup>3</sup> (lb/ft<sup>3</sup>), determined in the lab

$P_c$  = percent of dry oversize particles, of sieve used, by weight.

$P_f$  = percent of dry fine particles, of sieve used, by weight.

$k$  = Metric: 1,000 \* Bulk Specific Gravity ( $G_{sb}$ ) (oven dry basis)  
of coarse particles (kg/m<sup>3</sup>).

$k$  = English: 62.4 \* Bulk Specific Gravity ( $G_{sb}$ ) (oven dry basis)  
of coarse particles (lb/ft<sup>3</sup>)

**Note 3:** If the specific gravity is known, then this value will be used in the calculation. For most construction activities, the specific gravity for aggregate may be assumed to be 2.600.

## Calculation

### Example

- Metric:

Maximum laboratory dry density ( $\rho_f$ ): 1880 kg/m<sup>3</sup>

Percent coarse particles ( $P_c$ ): 27%

Percent fine particles ( $P_f$ ): 73%

Mass per volume coarse particles ( $k$ ): (2.697) (1000) = 2697 kg/m<sup>3</sup>

$$\rho_d = \frac{100\%}{\left[\left(\frac{P_f}{\rho_f}\right) + \left(\frac{P_c}{k}\right)\right]}$$

$$\rho_d = \frac{100\%}{\left[\left(\frac{73\%}{1880 \text{ kg/m}^3}\right) + \left(\frac{27\%}{2697 \text{ kg/m}^3}\right)\right]}$$

$$\rho_d = \frac{100\%}{[0.03883 \text{ kg/m}^3 + 0.01001 \text{ kg/m}^3]}$$

$$\rho_d = 2047.5 \text{ kg/m}^3 \text{ report } 2048 \text{ kg/m}^3$$

- English:

Maximum laboratory dry density ( $\rho_f$ ): 117.3 lb/ft<sup>3</sup>

Percent coarse particles ( $P_c$ ): 27%

Percent fine particles ( $P_f$ ): 73%

Mass per volume coarse particles ( $k$ ): (2.697) (62.4) = 168.3 lb/ft<sup>3</sup>

16

$$\rho_d = \frac{100\%}{\left[\left(\frac{P_f}{\rho_f}\right) + \left(\frac{P_c}{k}\right)\right]}$$

$$\rho_d = \frac{100\%}{\left[\left(\frac{73\%}{117.3 \text{ lb/ft}^3}\right) + \left(\frac{27\%}{168.3 \text{ lb/ft}^3}\right)\right]}$$

17

$$\rho_d = \frac{100\%}{[0.6223 \text{ lb/ft}^3 + 0.1604 \text{ lb/ft}^3]}$$

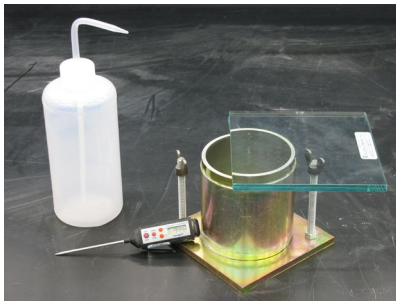
$$\rho_d = \frac{100\%}{0.7827 \text{ lb/ft}^3}$$

$$\rho_d = 127.76 \text{ lb/ft}^3 \quad \text{Report } 127.8 \text{ lb/ft}^3$$

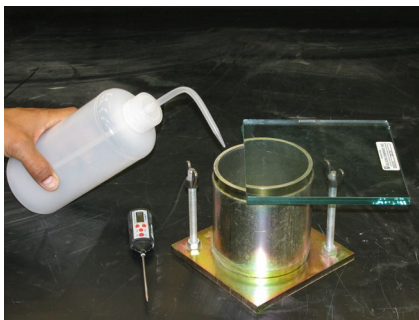
19

**Report**

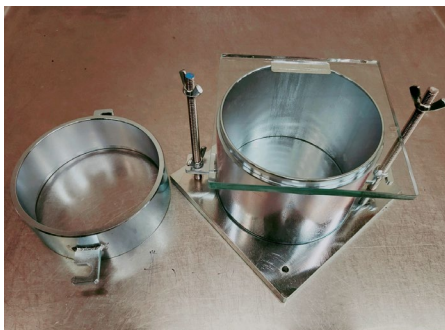
- Results on forms approved by the agency
- Sample ID
- Corrected maximum dry density to the nearest 1 kg/m<sup>3</sup> (0.1 lb/ft<sup>3</sup>)
- Corrected optimum moisture to the nearest 0.1 percent



**Apparatus**



**Filling the mold**



**Dried filled mold**

21

**ANNEX B  
STANDARDIZATION OF THE MOLD**

22

(Mandatory Information)

Standardization is a critical step to ensure accurate test results when using this apparatus. Failure to perform the standardization procedure as described herein will produce inaccurate or unreliable test results.

23

**Apparatus**

- Mold and base plate
- Balance or scale – Accurate to within 45 g (0.1 lb) or 0.3 percent of the test load, whichever is greater, at any point within the range of use.
- Cover plate – A piece of plate glass, at least 6 mm (1/4 in.) thick and at least 25 mm (1 in.) larger than the diameter of the mold.
- Thermometers – Standardized liquid-in-glass, or electronic digital total immersion type, accurate to 0.5°C (1°F)

24

25

**Procedure**

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1. Create a watertight seal between the mold and base plate.
2. Determine and record the mass of the dry sealed mold, base plate, and cover plate.
3. Fill the mold with water at a temperature between 16°C and 29°C (60°F and 85°F) and cover with the cover plate in such a way as to eliminate bubbles and excess water.
4. Wipe the outside of the mold, base plate, and cover plate dry, being careful not to lose any water from the mold.
5. Determine and record the mass of the filled mold, base plate, cover plate, and water.
6. Determine and record the mass of the water in the mold by subtracting the mass in Step 2 from the mass in Step 5.

7. Measure the temperature of the water and determine its density from Table B1, interpolating, as necessary.
8. Calculate the volume of the mold,  $V_m$ , by dividing the mass of the water in the mold by the density of the water at the measured temperature.

**Calculations**

31

$$V_m = \frac{M}{\rho_{water}}$$

Where:

$V_m$  = volume of the mold

$M$  = mass of water in the mold

$\rho_{water}$  = density of water at the measured temperature

**Example**

32

Mass of water in mold = 0.94367 kg (2.0800 lb)

$\rho_{water}$  at 23°C (73.4°F) = 997.54 kg/m<sup>3</sup> (62.274 lb/ft<sup>3</sup>)

$$V_m = \frac{0.94367 \text{ kg}}{997.54 \text{ kg/m}^3} = 0.000946 \text{ m}^3 \quad V_m = \frac{2.0800 \text{ lb}}{62.274 \text{ lb/ft}^3} = 0.0334 \text{ ft}^3$$

**Table B1**  
**Unit Mass of Water**  
**15°C to 30°C**

30

°C	(°F)	kg/m <sup>3</sup>	(lb/ft <sup>3</sup> )	°C	(°F)	kg/m <sup>3</sup>	(lb/ft <sup>3</sup> )
15	(59.0)	999.10	(62.372)	23	(73.4)	997.54	(62.274)
15.6	(60.0)	999.01	(62.366)	23.9	(75.0)	997.32	(62.261)
16	(60.8)	998.94	(62.361)	24	(75.2)	997.29	(62.259)
17	(62.6)	998.77	(62.350)	25	(77.0)	997.03	(62.243)
18	(64.4)	998.60	(62.340)	26	(78.8)	996.77	(62.227)
18.3	(65.0)	998.54	(62.336)	26.7	(80.0)	996.59	(62.216)
19	(66.2)	998.40	(62.328)	27	(80.6)	996.50	(62.209)
20	(68.0)	998.20	(62.315)	28	(82.4)	996.23	(62.192)
21	(69.8)	997.99	(62.302)	29	(84.2)	995.95	(62.175)
21.1	(70.0)	997.97	(62.301)	29.4	(85.0)	995.83	(62.166)
22	(71.6)	997.77	(62.288)	30	(86.0)	995.65	(62.156)

33

**Report**

- Mold ID
- Date Standardized
- Temperature of the water
- Volume,  $V_m$ , of the mold the nearest  $0.000001 \text{ m}^3$  ( $0.0001 \text{ ft}^3$ )





7. A soil-aggregate mixture has a maximum dry density of  $138.6 \text{ lb/ft}^3$  English units and optimum moisture of 6.4 percent. The coarse particles make up 22 percent of the material, having a  $G_{sb}$  of 2.631 and 1.7 percent moisture.

What is the corrected maximum density?

What is the corrected moisture?

**PERFORMANCE EXAM CHECKLIST**

**MOISTURE-DENSITY RELATION OF SOILS  
FOP FOR AASHTO T 99**

Participant Name: \_\_\_\_\_ Exam Date: \_\_\_\_\_

Record the symbols “P” for passing or “F” for failing on each step of the checklist.

<b>Procedure Element</b>	<b>Trial 1</b>	<b>Trial 2</b>
1. If damp, sample dried in air or drying apparatus, not exceeding 60°C (140°F)?	_____	_____
2. Sample broken up and an adequate amount sieved over the appropriate sieve (4.75 mm / No. 4 or 19.0 mm / 3/4 in.) to determine oversize (coarse particle) percentage?	_____	_____
3. Sample passing the sieve has appropriate mass?	_____	_____
4. If material is degradable:		
a. Multiple samples mixed with water varying moisture content by 1 to 2 percent, bracketing the optimum moisture content?	_____	_____
5. If soil is plastic (clay types):		
a. Multiple samples mixed with water varying moisture content by 1 to 2 percent, bracketing the optimum moisture content?	_____	_____
b. Samples placed in covered containers and allowed to stand for at least 12 hours?	_____	_____
6. Sample determined to be 4 to 8 percent below expected optimum moisture content?	_____	_____
7. Determine mass of clean, dry mold without collar to nearest 1 g (0.005 lb.)?	_____	_____
8. Mold placed on rigid and stable foundation?	_____	_____
9. Layer of soil (approximately one third compacted depth) placed in mold with collar attached, loose material lightly tamped?	_____	_____
10. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
11. Material adhering to the inside of the mold trimmed?	_____	_____
12. Layer of soil (approximately two thirds compacted depth) placed in mold with collar attached, loose material lightly tamped?	_____	_____
13. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
14. Material adhering to the inside of the mold trimmed?	_____	_____
15. Mold filled with soil such that compacted soil will be above the mold, loose material lightly tamped?	_____	_____

**OVER**

Procedure Element	Trial 1	Trial 2
16. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
17. Collar removed without shearing off sample?	_____	_____
18. Approximately 6 mm (1/4 in.) of compacted material above the top of the mold (without the collar)?	_____	_____
19. Soil trimmed to top of mold with the beveled side of the straightedge?	_____	_____
20. Remove all soil from exterior surface of mold and base plate?	_____	_____
21. Mass of mold and contents determined to nearest 1 g (0.005 lb.)?	_____	_____
22. Wet density calculated from the wet mass?	_____	_____
23. Soil removed from mold using a sample extruder if needed?	_____	_____
24. Soil sliced vertically through center (non-granular material)?	_____	_____
25. Moisture sample removed ensuring all layers are represented?	_____	_____
26. Moist mass determined immediately to 0.1 g?	_____	_____
27. Moisture sample mass of correct size?	_____	_____
28. Sample dried, and water content determined according to the FOP for T 255/T 265?	_____	_____
29. Remainder of material from mold broken up until it will pass through the sieve, as judged by eye, and added to remainder of original test sample?	_____	_____
30. Water added to increase moisture content of the remaining sample in approximately 1 to 2 percent increments?	_____	_____
31. Steps 7 through 29 repeated for each increment of water added?	_____	_____
32. Process continued until wet density either decreases or stabilizes?	_____	_____
33. Moisture content and dry density calculated for each sample?	_____	_____
34. Dry density plotted on vertical axis, moisture content plotted on horizontal axis, and points connected with a smooth curve?	_____	_____
35. Moisture content at peak of curve recorded as optimum water content and recorded to nearest 0.1 percent?	_____	_____
36. Dry density at optimum moisture content reported as maximum density to nearest 1 kg/m <sup>3</sup> (0.1 lb/ft <sup>3</sup> )?	_____	_____
37. Corrected for coarse particles if applicable?	_____	_____

Comments: First attempt: Pass\_\_\_\_\_Fail\_\_\_\_\_ Second attempt: Pass\_\_\_\_\_Fail\_\_\_\_\_

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Examiner Signature \_\_\_\_\_ WAQTC #: \_\_\_\_\_

**PERFORMANCE EXAM CHECKLIST**

**MOISTURE-DENSITY RELATION OF SOILS  
FOP FOR AASHTO T 180**

Participant Name: \_\_\_\_\_ Exam Date: \_\_\_\_\_

Record the symbols “P” for passing or “F” for failing on each step of the checklist.

<b>Procedure Element</b>	<b>Trial 1</b>	<b>Trial 2</b>
1. If damp, sample dried in air or drying apparatus, not exceeding 60°C (140°F)?	_____	_____
2. Sample broken up and an adequate amount sieved over the appropriate sieve (4.75 mm / No. 4 or 19.0 mm / 3/4 in.) to determine oversize (coarse particle) percentage?	_____	_____
3. Sample passing the sieve has appropriate mass?	_____	_____
4. If material is degradable:		
a. Multiple samples mixed with water varying moisture content by 1 to 2 percent, bracketing the optimum moisture content?	_____	_____
5. If soil is plastic (clay types):		
a. Multiple samples mixed with water varying moisture content by 1 to 2 percent, bracketing the optimum moisture content?	_____	_____
b. Samples placed in covered containers and allowed to stand for at least 12 hours?	_____	_____
6. Sample determined to be 4 to 8 percent below expected optimum moisture content?	_____	_____
7. Determine mass of clean, dry mold without collar to nearest 1 g (0.005 lb.)?	_____	_____
8. Mold placed on rigid and stable foundation?	_____	_____
9. Layer of soil (approximately one fifth compacted depth) placed in mold with collar attached, loose material lightly tamped?	_____	_____
10. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
11. Material adhering to the inside of the mold trimmed?	_____	_____
12. Layer of soil (approximately two fifths compacted depth) placed in mold with collar attached, loose material lightly tamped?	_____	_____
13. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
14. Material adhering to the inside of the mold trimmed?	_____	_____
15. Layer of soil (approximately three fifths compacted depth) placed in mold with collar attached, loose material lightly tamped?	_____	_____

**OVER**

<b>Procedure Element</b>	<b>Trial 1</b>	<b>Trial 2</b>
16. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
17. Material adhering to the inside of the mold trimmed?	_____	_____
18. Layer of soil (approximately four fifths compacted depth) placed in mold with collar attached, loose material lightly tamped?	_____	_____
19. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
20. Material adhering to the inside of the mold trimmed?	_____	_____
21. Mold filled with soil such that compacted soil will be above the mold, loose material lightly tamped?	_____	_____
22. Soil compacted with appropriate number of blows (25 or 56)?	_____	_____
23. Collar removed without shearing off sample?	_____	_____
24. Approximately 6 mm (1/4 in.) of compacted material above the top of the mold (without the collar)?	_____	_____
25. Soil trimmed to top of mold with the beveled side of the straightedge?	_____	_____
26. Remove all soil from exterior surface of mold and base plate?	_____	_____
27. Mass of mold and contents determined to nearest 1 g (0.005 lb.)?	_____	_____
28. Wet density calculated from the wet mass?	_____	_____
29. Soil removed from mold using a sample extruder if needed?	_____	_____
30. Soil sliced vertically through center (non-granular material)?	_____	_____
31. Moisture sample removed ensuring all layers are represented?	_____	_____
32. Moist mass determined immediately to 0.1 g?	_____	_____
33. Moisture sample mass of correct size?	_____	_____
34. Sample dried, and water content determined according to the FOP for T 255/T 265?	_____	_____
35. Remainder of material from mold broken up until it will pass through the sieve, as judged by eye, and added to remainder of original test sample?	_____	_____
36. Water added to increase moisture content of the remaining sample in approximately 1 to 2 percent increments?	_____	_____
37. Steps 2 through 20 repeated for each increment of water added?	_____	_____
38. Process continued until wet density either decreases or stabilizes?	_____	_____
39. Moisture content and dry density calculated for each sample?	_____	_____
40. Dry density plotted on vertical axis, moisture content plotted on horizontal axis, and points connected with a smooth curve?	_____	_____

**OVER**

**Procedure Element**

**Trial 1 Trial 2**

- 41. Moisture content at peak of curve recorded as optimum water content and recorded to nearest 0.1 percent? \_\_\_\_\_
- 42. Dry density at optimum moisture content reported as maximum density to nearest 1 kg/m<sup>3</sup> (0.1 lb/ft<sup>3</sup>)? \_\_\_\_\_
- 43. Corrected for coarse particles if applicable? \_\_\_\_\_

Comments: First attempt: Pass \_\_\_\_\_ Fail \_\_\_\_\_ Second attempt: Pass \_\_\_\_\_ Fail \_\_\_\_\_

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Examiner Signature \_\_\_\_\_ WAQTC #: \_\_\_\_\_



**DEVELOPING SOIL MOISTURE-DENSITY RELATIONS(FAMILY OF CURVES)  
FOP FOR AASHTO R 75**

01

**Significance**

02

Soils sampled from one source will have many different moisture-density curves, but if a group of these curves is plotted together, similarities are usually seen. A group of soil moisture-density relations have similarities characteristic of the soil type and source. Higher-density soils have curves with steeper slopes and maximum dry densities at lower optimum moisture contents, while the lower-density soils have flatter curves with higher optimum moisture contents.

03

**Scope**

04

This procedure provides a method to develop moisture-density relations using multiple moisture-density curves developed using the same method, A, B, C, or D from the FOP for AASHTO T 99/T 180 in accordance with AASHTO R 75-24.

All curves used in a group must be developed using a single Method: A, B, C, or D of a procedure for AASHTO T 99 or T 180. See the FOP for AASHTO T 99/T 180.

05

**Terminology**

*soil moisture-density relations* — a group of soil moisture-density relation (commonly known as a family of curves) determined using AASHTO T 99 or T 180, which reveal certain similarities and trends characteristic of the soil type and source.

*spine* — smooth line extending through the point of maximum density and optimum moisture content of a group of moisture-density curves.

**Procedure**

06

1. Sort the curves by Method (A, B, C, or D of the FOP for T 99/T 180). At least three curves are required per group.
2. Select the highest and lowest maximum dry densities from those selected to assist in

Mold \ Sieve	4.75 mm (No. 4)	19 mm (3/4")
4"	A	C
6"	B	D

**Methods of T 99 / T 180**

- determining the desired scale of the subsequent graph.
- 07 3. Plot the maximum density and optimum moisture points of the selected curves on the graph.
  - 08 4. Draw a smooth, “best fit,” curved line through the points creating the spine of the soil moisture-density relations.
  - 09
  - 10 5. Remove maximum density and optimum moisture points that were not used to establish the spine.
  - 11 6. Add the moisture-density curves associated with the points that were used to establish the spine. It is not necessary to include the portion of the curves over optimum moisture.
  - 12
  - 13 *Note 1*—Intermediate curves using slopes similar to those of the original moisture-density curves may be included when maximum density points are more than 2.0 lb/ft<sup>3</sup> apart. Intermediate curves are indicated by a dashed line.
  - 14
  - 15
  - 16 7. Plot the 80 percent of optimum moisture range when desired:
    - a. Using the optimum moisture of an existing curve, calculate 80 percent of optimum moisture and plot this value on the curve. Repeat for each curve in the group.
    - b. Draw a smooth, “best fit,” curved line connecting the 80 percent of optimum moisture points plotted on the curves that parallel the spine.

**Calculations**

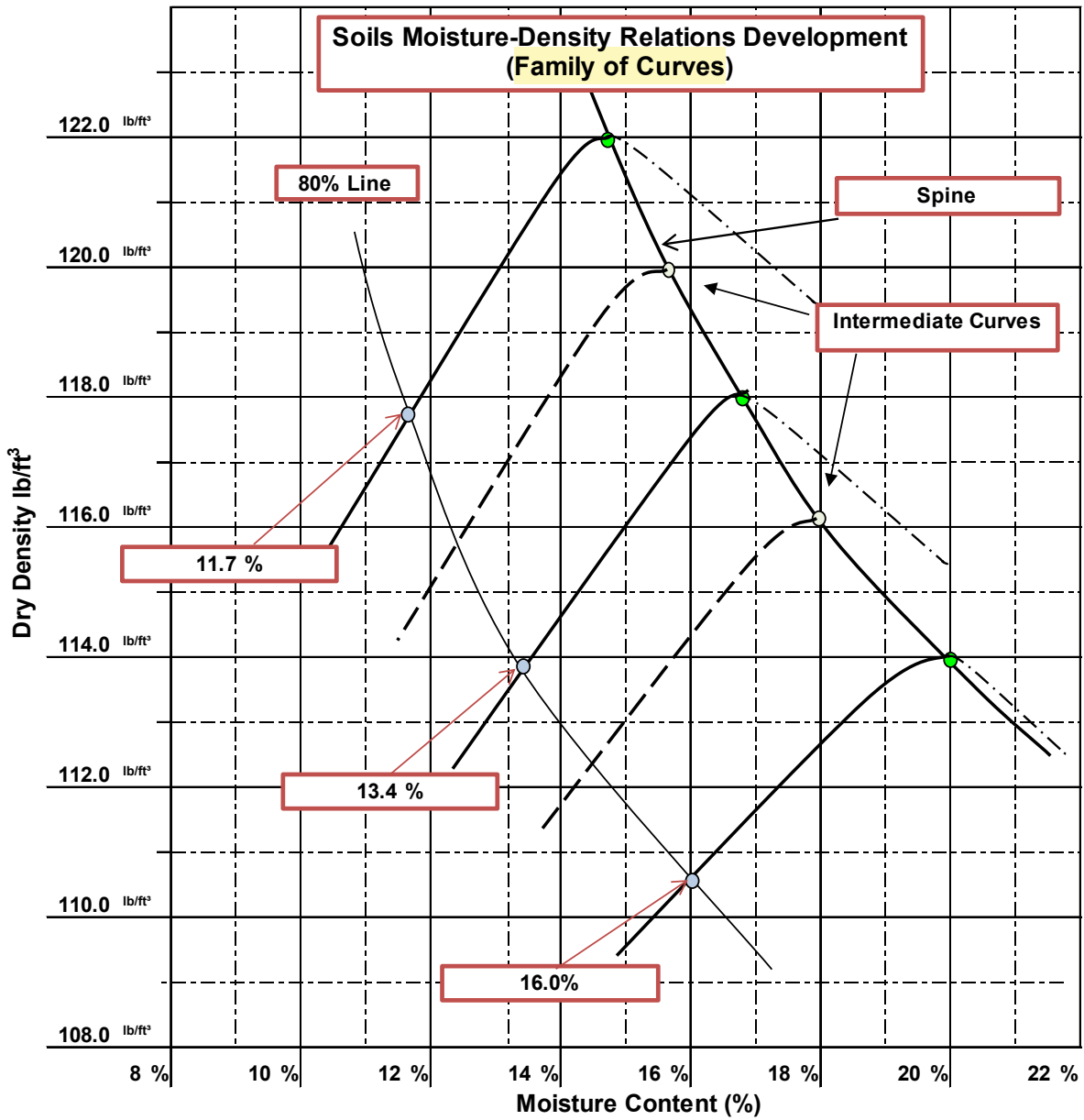
Calculate 80 percent of optimum moisture of each curve:

17

Example:

Optimum moisture of the highest density curve = 14.6%

$$80\% \text{ of optimum moisture} = \frac{80}{100} \times 14.6\% = 11.7\%$$



**Tips!**

19

- Make sure that the selected moisture-density relations curves were developed using the same method from the FOP for AASHTO T 99 / T 180 – A, B, C, or D

**REVIEW QUESTIONS**

1. To what other procedure(s) is this procedure related?
2. What does the 'spine' of the group mean?
3. Describe the limitations of developing moisture-density relations.

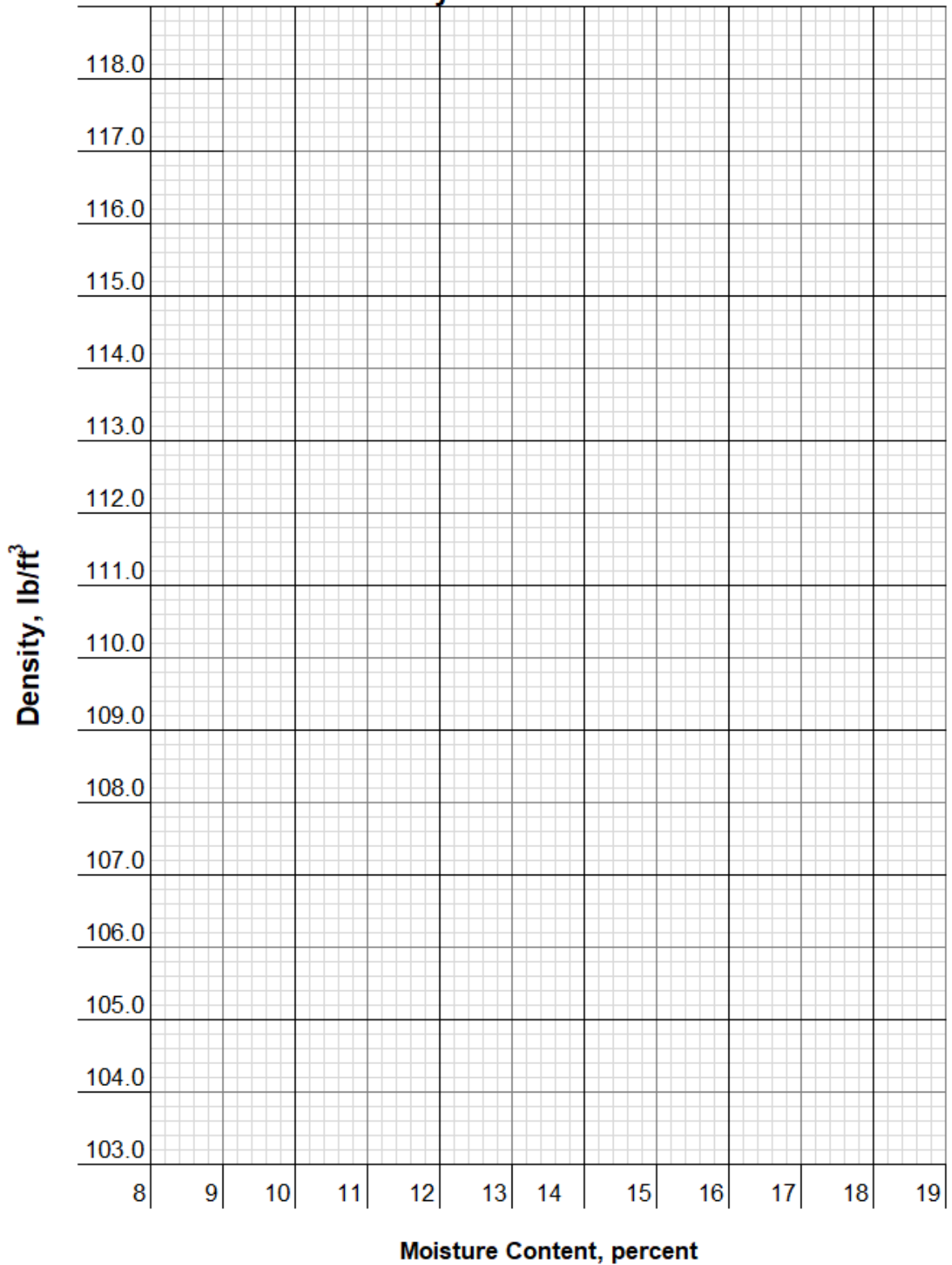


## STEPS FOR DEVELOPING SOIL MOISTURE-DENSITY RELATIONS (FAMILY OF CURVES)

1. Sort the curves by Test Procedure.
2. Sort the curves by Method.
3. Select a group of curves with three or more.
4. Determine the scale of the graph.
  - a. For the vertical axis, identify the highest maximum dry density and the lowest point from the lowest dry density curve.
  - b. For the horizontal axis, identify the lowest and highest percent moisture contents of the points of the curves.
5. Plot the maximum dry density and optimum moisture points of the selected curves on the graph.
6. Do at least three curves 'line up' to draw a 'spine?'
  - a. Draw a smooth, 'best fit,' curved line through the points that line up creating the spine of the family of curves.
7. Remove the point of the curves that don't fit the spine.
8. Plot the full curve of the points on the spine (points over optimum moisture are not required).
9. Are intermediate template curves desired?
  - a. Maximum dry density points more than 2.0 lb/ft<sup>3</sup> apart
  - b. Draw template curves with dashed lines using a similar slope as the original curves
10. Plot the 80 percent of optimum moisture range when desired
  - a. Using the optimum moisture of the existing curve, calculate 80 percent of optimum moisture and plot this value on the curve. Repeat for each curve in the group.
  - b. Draw a smooth, "best fit," curved line connecting the 80 percent of optimum moisture points plotted on the curves that parallel the spine.

Moisture – Density Curve ID	Dry Density lb/ft <sup>3</sup>	Dry Density lb/ft <sup>3</sup>	Dry Density lb/ft <sup>3</sup>	Maximum Dry Density lb/ft <sup>3</sup>	Dry Density lb/ft <sup>3</sup>	Dry Density lb/ft <sup>3</sup>
	Moisture Content, %	Moisture Content, %	Moisture Content, %	Optimum Moisture, %	Moisture Content, %	Moisture Content, %
#1 T 99 A	104.4	105.1	105.8	106.2	106.0	104.2
	14.1	14.8	15.8	17.1	18.1	20.1
#2 T 180 A	106.2	106.9	107.6	108.0	107.8	106.0
	13.3	14.1	15.1	16.4	17.4	19.4
#3 T 180 A	111.2	112.4	113.6	114.0	113.5	111.4
	10.4	10.9	11.6	12.5	13.2	14.7
#4 T 99 A	109.4	110.6	111.8	112.2	111.7	109.6
	11.1	11.6	12.3	13.2	13.9	15.4
#5 T 99 C	112.1	113.2	114.7	115.4	114.7	113.0
	9.8	10.3	11.0	11.8	13.2	13.5
#6 T 180 A	111.4	114.2	116.5	117.2	116.5	114.8
	8.2	9.2	10.3	11.1	12.0	12.8
#7 T 180 A	112.8	115.0	116.6	117.2	116.2	114.4
	13.0	13.6	14.0	14.5	15.2	15.9
#8 T 180 A	105.0	107.2	108.5	109.0	107.6	105.2
	8.3	9.0	9.5	10.1	11.2	12.2
#9 T 180 A	106.6	108.4	110.2	110.6	110.1	108.4
	10.5	11.6	13.2	14.3	15.6	17.4
#10 T 99 C	111.0	113.2	114.8	115.4	114.4	112.6
	13.7	14.3	14.7	15.2	15.9	16.5

### Soil Moisture-Density Relations (Family of Curves) Graph





**PERFORMANCE EXAM CHECKLIST**

**DEVELOPING SOIL MOISTURE-DENSITY RELATIONS (FAMILY OF CURVES)  
FOP FOR AASHTO R 75**

Participant Name \_\_\_\_\_ Exam Date \_\_\_\_\_

Record the symbols "P" for passing or "F" for failing on each step of the checklist.

<b>Procedure Element</b>	<b>Trial 1</b>	<b>Trial 2</b>
1. Curves sorted by method and procedure (A, B, C, or D of the FOP for T 99/T 180)?	_____	_____
a. At least three curves per group?	_____	_____
b. Curves within a group are similar soil type and from same source?	_____	_____
2. Maximum density and optimum moisture points plotted on the graph?	_____	_____
3. Spine drawn correctly?	_____	_____
4. Maximum density and optimum moisture points removed that were not used for the spine?	_____	_____
5. Moisture-density curves added?	_____	_____
6. Optimum moisture range?	_____	_____
a. 80 percent of optimum moisture calculated for each curve?	_____	_____
b. Curved line through 80 percent of optimum moisture drawn correctly?	_____	_____

Comments: First attempt: Pass \_\_\_\_\_ Fail \_\_\_\_\_ Second attempt: Pass \_\_\_\_\_ Fail \_\_\_\_\_

\_\_\_\_\_

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\_\_\_\_\_

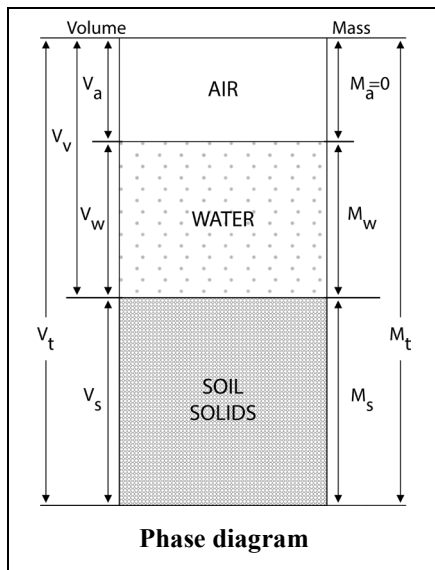
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Examiner Signature \_\_\_\_\_ WAQTC #: \_\_\_\_\_



**SPECIFIC GRAVITY AND ABSORPTION OF COARSE AGGREGATE  
FOP FOR AASHTO T 85**



01  
02  
03

**Significance**

Bulk specific gravity is a characteristic used for calculating the volume occupied by the aggregate or various mixtures containing aggregate, including Portland Cement Concrete, asphalt mixtures, and other materials that are proportioned or analyzed on an absolute volume basis. Specific gravity is the ratio of the mass of a material to the mass of an equal volume of water. Several categories of specific gravity are used relative to aggregate.

Bulk specific gravity (oven dry),  $G_{sb}$ , is used for computations when the aggregate is dry. Bulk specific gravity (saturated surface dry or SSD),  $G_{sb}$  SSD, is used if the aggregate is wet. Apparent specific gravity,  $G_{sa}$ , is based solely on the solid material making up the constituent particles and does not include the pore space within the particles that is accessible to water.

Absorption values are used to calculate the change in the mass of an aggregate due to water absorbed in the pore spaces within the constituent particles, compared to the dry condition, when it is deemed that the aggregate has been in contact with water long enough to satisfy most of the absorption potential. The laboratory standard for absorption is that obtained after submerging dry aggregate for between 15 to 19 hours in water. Aggregates mined from below the water table may have a higher absorption, when used, if not allowed to dry. Conversely, some aggregates, when used, may contain an amount of absorbed moisture less than the 15-hour-soaked condition. For an aggregate that has been in contact with water and that has free moisture on the particle surfaces, the percentage of free moisture can be determined by deducting the absorption from the total moisture content.

The pores in lightweight aggregates may or may not become filled with water after immersion for 15 hours. In fact, many such aggregates can remain immersed in water for several days without satisfying most of the aggregates' absorption

potential. Therefore, this method is not intended for use with lightweight aggregate.

**Scope**

This procedure covers the determination of specific gravity and absorption of coarse aggregate in accordance with AASHTO T 85-25. Specific gravity may be expressed as bulk specific gravity ( $G_{sb}$ ), bulk specific gravity - saturated surface dry ( $G_{sb SSD}$ ), or apparent specific gravity ( $G_{sa}$ ).  $G_{sb}$  and absorption are based on aggregate after soaking in water. This procedure is not intended for use with lightweight aggregates.

**Terminology**

Absorption – the increase in the mass of aggregate due to water being absorbed into the pores of the material but not including water adhering to the outside surface of the particles, expressed as a percentage of the dry mass. Saturated Surface Dry (SSD) – the condition of an aggregate particle when the permeable voids are filled with water, but no water is present on exposed surfaces.

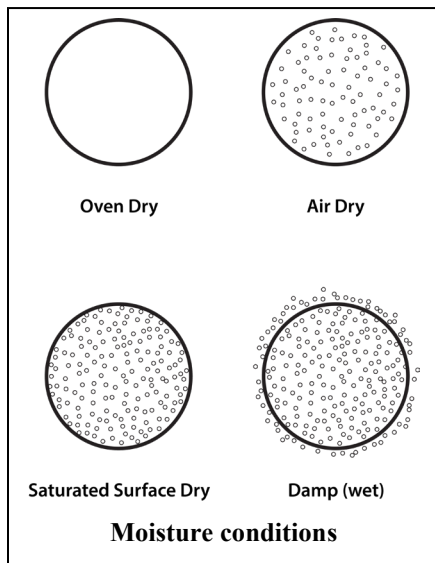
Specific Gravity – the ratio of the mass, in air, of a volume of a material to the mass of the same volume of gas-free distilled water at a stated temperature.

Apparent Specific Gravity ( $G_{sa}$ ) – the ratio of the mass, in air, of a volume of the impermeable portion of aggregate to the mass of an equal volume of gas-free distilled water at a stated temperature.

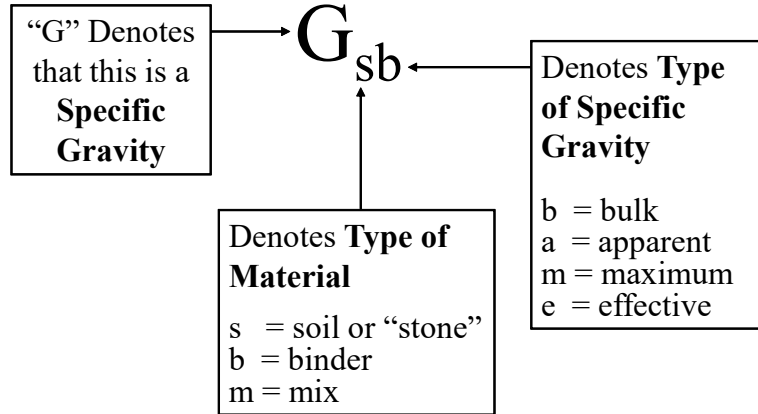
Bulk Specific Gravity ( $G_{sb}$ ) – the ratio of the mass, in air, of a volume of aggregate (including the permeable and impermeable voids in the particles but not including the voids between particles) to the mass of an equal volume of gas-free distilled water at a stated temperature.

Bulk Specific Gravity (SSD) ( $G_{sb SSD}$ ) – the ratio of the mass, in air, of a volume of aggregate, including the mass of water within the voids filled to the extent achieved by submerging in water for 15 to 19 hours (but not including the voids between particles), to the mass of an equal volume of gas-free distilled water at a stated temperature.

04  
05  
06  
07



## Definition: (Specific Gravity Symbols)



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### Apparatus

- Balance or scale: with a capacity of 5 kg, sensitive to 0.1 g, and meeting the requirements of AASHTO M 231.
- Sample container: a wire basket of 3.35 mm (No. 6) or smaller mesh, with a capacity of 4 to 7 L (1 to 2 gal) to contain aggregate with a nominal maximum size of 37.5 mm (1 1/2 in.) or smaller; or a larger basket for larger aggregates, or both.
- Water tank: watertight and large enough to completely immerse aggregate and basket, equipped with an overflow valve to keep water level constant.
- Suspension apparatus: wire used to suspend apparatus shall be of the smallest practical diameter.
- Sieves: 4.75 mm (No. 4) or other sizes as needed, meeting the requirements of FOP for AASHTO T 27/T 11.
- Large absorbent cloth

**Sample Preparation**

- 11 1. Obtain the sample in accordance with the FOP for AASHTO R 90 (see Note 1).
- 2. Mix the sample thoroughly and reduce to the approximate sample size required by Table 1 in accordance with the FOP for AASHTO R 76.
- 3. Reject all material passing the appropriate sieve by dry sieving.
- 4. Thoroughly wash the sample to remove dust or other coatings from the surface.
- 12 5. Dry the sample to constant mass according to the FOP for AASHTO T 255/T 265 at a temperature of 110 ±5°C (230 ±9°F) and cool in air at room temperature for 1 to 3 hours.

*Note 1:* Where the absorption and specific gravity values are to be used in proportioning concrete mixtures in which the aggregates will be in their naturally moist condition, the requirement for initial drying to constant mass may be eliminated, and, if the surfaces of the particles in the sample have been kept continuously wet until test, the 15-to-19-hour soaking may also be eliminated.

- 6. Re-screen the sample over the appropriate sieve. Reject all material passing that sieve.
- 7. The sample shall meet or exceed the minimum mass given in Table 1.

*Note 2:* If this procedure is used only to determine the G<sub>sb</sub> of oversized material for the FOP for AASHTO T 99 or T 180, the material can be rejected over the appropriate sieve. For T 99 / T 180 Methods A and B, use the 4.75 mm (No.4) sieve; for T 99 / T 180 Methods C and D, use the 19 mm (3/4 in).

**Table 1**

Nominal Maximum Size*, mm (in.)	Minimum Mass of Sample, g (lb)
12.5 (1/2) or less	2000 (4.4)
19.0 (3/4)	3000 (6.6)
25.0 (1)	4000 (8.8)
37.5 (1 1/2)	5000 (11)
50 (2)	8000 (18)
63 (2 1/2)	12,000 (26)
75 (3)	18,000 (40)

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\* One sieve larger than the first sieve to retain more than 10 percent of the material using an agency specified set of sieves based on cumulative percent retained. Where large gaps in specification sieves exist, intermediate sieve(s) may be inserted to determine nominal maximum size.

**Procedure**

1. Immerse the sample in water at room temperature for a period of 15 to 19 hours.

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*Note 3:* When testing coarse aggregate of large nominal maximum size requiring large samples, it may be more convenient to perform the test on two or more subsamples, and then combine values obtained.

2. Place the empty basket into the water bath and attach to the balance. Inspect the immersion tank to ensure the water level is at the overflow outlet height and basket is fully submerged. Tare the balance with the empty basket attached in the water bath.

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3. Remove the sample from the water and roll it in a large absorbent cloth until all visible films of water are removed. Wipe the larger particles individually. If the sample dries past the SSD condition, immerse in water for 30 min, and then resume the process of surface-drying.

*Note 4:* A moving stream of air may be used to assist in the drying operation but take care to avoid evaporation of water from aggregate pores.

4. Determine the SSD mass of the sample, and record this and all subsequent masses to the nearest 0.1 g or 0.1 percent of the sample mass,



**SSD Sample**

whichever is greater. Designate this mass as “B.”

5. Immediately place the SSD sample in the sample container and weigh it in water maintained at  $23.0 \pm 1.7^{\circ}\text{C}$  ( $73.4 \pm 3^{\circ}\text{F}$ ). Shake the container to release entrapped air before recording the weight. Re-inspect the immersion tank to ensure the water level is at the overflow outlet height and basket is fully submerged. Designate this submerged weight as “C.”

*Note 5:* The container should be immersed to a depth sufficient to cover both it and the sample during mass determination. The wire suspending the container should be of the smallest practical size to minimize any possible effects of a variable immersed length.



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6. Remove the sample from the basket. Ensure that all material has been removed and place in a container of known mass.
7. Dry the sample to constant mass according to the FOP for AASHTO T 255/T 265 at  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ) and cool in air at room temperature for 1 to 3 hours.
8. Determine and record the dry mass. Designate this mass as “A.”

**Calculations**

Perform calculations and determine values using the appropriate formula below.

Bulk specific gravity ( $G_{sb}$ )

$$G_{sb} = \frac{A}{B - C} \quad 18$$

Bulk specific gravity, SSD ( $G_{sb\text{ SSD}}$ )

$$G_{sb\text{ SSD}} = \frac{B}{B - C} \quad 19$$

Apparent specific gravity ( $G_{sa}$ )

$$G_{sa} = \frac{A}{A - C} \quad 20$$

Absorption

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$$\text{Absorption} = \frac{B - A}{A} \times 100$$

Where:

- A = oven dry mass, g
- B = SSD mass, g
- C = weight in water, g

**Sample Calculations**

Sample	A	B	C	B - C	A - C	B - A
1	2030.9	2044.9	1304.3	740.6	726.6	14.0
2	1820.0	1832.5	1168.1	664.4	651.9	12.5
3	2035.2	2049.4	1303.9	745.5	731.3	14.2

Sample	G <sub>sb</sub>	G <sub>sb</sub> SSD	G <sub>sa</sub>	Absorption
1	2.742	2.761	2.795	0.7
2	2.739	2.758	2.792	0.7
3	2.730	2.749	2.783	0.7

These calculations demonstrate the relationship between G<sub>sb</sub>, G<sub>sb</sub> SSD, and G<sub>sa</sub>. G<sub>sb</sub> is always lowest since the volume includes voids permeable to water. G<sub>sb</sub> SSD is always intermediate. G<sub>sa</sub> is always highest since the volume does not include voids permeable to water. When running this test, check to make sure the values calculated make sense in relation to one another.

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**Report**

- On forms approved by the agency
- Sample ID
- Specific gravities to the nearest 0.001
- Absorption to the nearest 0.1 percent

**Tips!**

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- Shake the container and sample when weighing in water to release entrapped air.
- Compare G<sub>sb</sub>, G<sub>sb</sub> SSD, and G<sub>sa</sub> to see if they make sense.

**REVIEW QUESTIONS**

1. What size sample is required for aggregate with a nominal maximum size of 25 mm (1 in.)?
2. When is soaking required? For how long must material be soaked?
3. When, in the process, are dry and SSD masses determined?



**PERFORMANCE EXAM CHECKLIST**

**SPECIFIC GRAVITY AND ABSORPTION OF COARSE AGGREGATE  
FOP FOR AASHTO T 85**

Participant Name \_\_\_\_\_ Exam Date \_\_\_\_\_

Record the symbols “P” for passing or “F” for failing on each step of the checklist.

Procedure Element	Trial 1	Trial 2
1. Sample obtained by FOP for AASHTO R 90 and reduced by FOP for AASHTO R 76 or from FOP for AASHTO T 99 / T 180?	_____	_____
2. Screened on the appropriate size sieve?	_____	_____
3. Sample mass appropriate?	_____	_____
4. Particle surfaces clean?	_____	_____
5. Dried to constant mass $110 \pm 5^{\circ}\text{C}$ ( $230 \pm 9^{\circ}\text{F}$ ) and cooled to room temperature?	_____	_____
6. Re-screen over appropriate sieve?	_____	_____
7. Covered with water for 15 to 19 hours?	_____	_____
8. Wire basket completely submerged in immersion tank and attached to balance?	_____	_____
9. Immersion tank inspected for proper water height?	_____	_____
10. Balance tared with basket in tank and temperature checked $23.0 \pm 1.7^{\circ}\text{C}$ ( $73.4 \pm 3^{\circ}\text{F}$ )?	_____	_____
11. Sample removed from water and rolled in cloth to remove visible films of water?	_____	_____
12. Larger particles wiped individually?	_____	_____
13. Evaporation avoided?	_____	_____
14. Sample mass determined to 0.1 g?	_____	_____
15. Sample immediately placed in basket, in immersion tank?	_____	_____
16. Entrapped air removed before weighing by shaking basket while immersed?	_____	_____
17. Immersion tank inspected for proper water height?	_____	_____
18. Immersed sample weight determined to 0.1 g?	_____	_____
19. All the sample removed from basket?	_____	_____
20. Sample dried to constant mass and cooled to room temperature?	_____	_____

**OVER**

**Procedure Element**

**Trial 1 Trial 2**

21. Sample mass determined to 0.1 g?

\_\_\_\_\_

22. Proper formulas used in calculations?

\_\_\_\_\_

Comments: First attempt: Pass\_\_\_\_\_Fail\_\_\_\_\_ Second attempt: Pass\_\_\_\_\_Fail\_\_\_\_\_

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Examiner Signature \_\_\_\_\_ WAQTC #: \_\_\_\_\_

**APPENDIX A  
FIELD OPERATING PROCEDURES – SHORT FORM**

<b><u>Chapter</u></b>	<b><u>Section</u></b>
7	FOP for AASHTO T 255 Total Evaporable Moisture Content of Aggregate by Drying AASHTO T 265 Laboratory Determination of Moisture Content of Soils
8	FOP for AASHTO T 99 Moisture-Density Relations of Soils Using a 2.5-kg (5.5-lb) Rammer and 305-mm (12-in.) Drop AASHTO T 180 Moisture-Density Relations of Soils Using a 4.54-kg (10-lb) Rammer and 457-mm (18-in.) Drop
9	FOP for AASHTO R 75 Developing Soil Moisture-Density Relations (Family of Curves)
10	FOP for AASHTO T 85 Specific Gravity and Absorption of Coarse Aggregate



**TOTAL EVAPORABLE MOISTURE CONTENT OF AGGREGATE BY DRYING  
FOP FOR AASHTO T 255  
LABORATORY DETERMINATION OF MOISTURE CONTENT OF SOILS  
FOP FOR AASHTO T 265**

### Scope

This procedure covers the determination of moisture content of aggregate and soil in accordance with AASHTO T 255-25 and AASHTO T 265-22. It may also be used for other construction materials.

### Overview

Moisture content is determined by comparing the wet mass of a sample and the mass of the sample after drying to constant mass. The term constant mass is used to define when a sample is dry.

*Constant mass* – the state at which a mass does not change more than a given percent, after additional drying for a defined time interval, at a required temperature.

### Apparatus

- Balance or scale: capacity sufficient for the principal sample mass, readable to 0.1 percent of sample mass or 0.1 g, and meeting the requirements of AASHTO M 231
- Containers: clean, dry, and capable of being sealed
- Suitable drying container
  - For soils: container requires close-fitting lid
  - For aggregate: container lid is optional
- Microwave safe container with ventilated lid (for drying aggregate only)
- Heat source, thermostatically controlled, capable of maintaining  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ).
  - Forced draft oven (preferred)
  - Ventilated oven
  - Convection oven
- Heat source, uncontrolled, for use when allowed by the agency, which will not alter the material being dried, and when close control of the temperature is not required:
  - Infrared heater/heat lamp, hot plate, fry pan, or any other device/method allowed by the agency .
  - Microwave oven (900 watts minimum)
- Utensils such as spoons
- Hot pads or gloves

**Sample Preparation**

Obtain a representative sample according to the FOP for AASHTO R 90 in its existing condition. If necessary, reduce the sample to moisture content sample size according to the FOP for AASHTO R 76.

For aggregate, the moisture content sample size is based on Table 1 or other information that may be specified by the agency.

**TABLE 1**  
**Sample Sizes for Moisture Content of Aggregate**

<b>Nominal Maximum Size* mm (in.)</b>	<b>Minimum Sample Mass g (lb)</b>
150 (6)	50,000 (110)
100 (4)	25,000 (55)
90 (3 1/2)	16,000 (35)
75 (3)	13,000 (29)
63 (2 1/2)	10,000 (22)
50 (2)	8000 (18)
37.5 (1 1/2)	6000 (13)
25.0 (1)	4000 (9)
19.0 (3/4)	3000 (7)
12.5 (1/2)	2000 (4)
9.5 (3/8)	1500 (3.3)
4.75 (No. 4)	500 (1.1)

\* One sieve larger than the first sieve to retain more than 10 percent of the material using an agency specified set of sieves based on cumulative percent retained. Where large gaps in specification sieves exist, intermediate sieve(s) may be inserted to determine nominal maximum.

For soils the moisture content sample size is based on Table 2 or other information that may be specified by the agency.

**TABLE 2**  
**Sample Sizes for Moisture Content of Soil**

<b>Maximum Particle Size mm (in.)</b>	<b>Minimum Sample Mass g</b>
50 (2)	1000
25.0 (1)	500
12.5 (1/2)	300
4.75 (No. 4)	100
0.425 (No. 40)	10

Immediately seal or cover moisture content samples to prevent any change in moisture content or follow the steps in “Procedure.”

### **Procedure**

Determine and record the sample masses as follows:

- For aggregate, determine and record all masses to the nearest 0.1 percent of the sample mass or to the nearest 0.1 g.
- For soil, determine and record all masses to the nearest 0.1 g.

When determining the mass of hot samples or containers or both, place and tare a buffer between the sample container and the balance. This will eliminate damage to or interference with the operation of the balance or scale.

1. Determine and record the mass of the container .
  - a. For aggregate: the lid is optional unless drying with a microwave then a ventilated lid is required.
  - b. For soils: the container includes the mass of the close-fitting lid.
2. Place the wet sample in the container.
3. Determine and record the total mass of the wet sample and container with lid, when used.
  - a. For oven(s), hot plates, infrared heaters, etc.: Spread the sample in the container.
  - b. For microwave oven: Heap sample in the container; cover with ventilated lid.
4. Subtract the container mass determined in Step 1 from the mass of the container and sample determined in Step 3 and record the wet mass of the sample ( $M_w$ ).
5. Remove the lid, if used, and place the sample in one of the following drying apparatuses:
  - a. For aggregate –
    - i. Controlled heat source (oven): at  $110 \pm 5^\circ\text{C}$  ( $230 \pm 9^\circ\text{F}$ ).
    - ii. Uncontrolled heat source (Hot plate, infrared heater, or other heat source as allowed by the agency): Stir frequently to avoid localized overheating.

b. For soil – controlled heat source (oven): at  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ).

**Note 1:** Soils containing gypsum or significant amounts of organic material require special drying. For reliable moisture contents dry these soils at  $60^{\circ}\text{C}$  ( $140^{\circ}\text{F}$ ). For more information see AASHTO T 265, Note 2.

6. Dry until sample appears moisture free.
7. Determine mass of sample and container and lid, when used.
8. Subtract the container mass determined in Step 1 from the mass of the container and sample determined in Step 7 and record the mass of the sample.
9. Return sample and container to the heat source for the additional time interval.
  - a. Drying intervals for aggregate –
    - i. Controlled heat source (oven): 30 minutes
    - ii. Uncontrolled heat source (Hot plate, infrared heater, or other heat source as allowed by the agency): 10 minutes
    - iii. Uncontrolled heat source (Microwave oven): 2 minutes

**Caution:** Some minerals in the sample may cause the aggregate to overheat, crack, and explode; altering the aggregate gradation.

**Table 3  
Methods of Drying**

<b>Aggregate</b>		
<b>Heat Source</b>	<b>Specific Instructions</b>	<b>Drying intervals to achieve constant mass (minutes)</b>
<b>Controlled:</b>		
Forced draft (preferred), ventilated, or convection oven	$110 \pm 5^{\circ}\text{C}$ ( $230 \pm 9^{\circ}\text{F}$ )	30
<b>Uncontrolled:</b>		
Hot plate, infrared heater, or any other device/method allowed by the agency	Stir frequently	10
Microwave	Heap sample and cover with ventilated lid	2

- b. Drying interval for soil – controlled heat source (oven): 1 hour

**Table 4**  
**Method of Drying**

<b>Soil</b>		
<b>Heat Source</b>	<b>Specific Instructions</b>	<b>Drying interval to achieve constant mass</b>
<b>Controlled:</b> Forced draft (preferred), ventilated, or convection oven	110 ±5°C (230 ±9°F)	1 hour

10. Determine mass of sample and container with lid, if used.
11. Subtract the container mass determined in Step 1 from the mass of the container and sample determined in Step 10 and record the mass of the sample.
12. Determine percent change by subtracting the new mass determination ( $M_n$ ) from the previous mass determination ( $M_p$ ), dividing by the previous mass determination ( $M_p$ ), and multiplying by 100.
13. Continue drying, performing steps 9 through 12, until there is less than a 0.10 percent change after additional drying time.
14. Constant mass has been achieved; sample is defined as dry.
15. Allow the sample and container with lid, when used, to cool. Determine and record the total mass.
16. Subtract the mass of the container determined in Step 1 from the mass of the container and sample determined in Step 15 and record the dry mass of the sample ( $M_D$ ).
17. Determine and record percent moisture ( $w$ ) by subtracting the final dry mass determination ( $M_D$ ) from the initial wet mass determination ( $M_W$ ), dividing by the final dry mass determination ( $M_D$ ), and multiplying by 100.

**Calculation****Constant Mass**

Calculate constant mass using the following formula:

$$\% \text{ Change} = \frac{M_p - M_n}{M_p} \times 100$$

Where:

$M_p$  = previous mass measurement

$M_n$  = new mass measurement

**Example:**

Mass of container: 1232.1 g

Mass of container and sample after first drying cycle: 2637.2 g

Mass,  $M_p$ , of possibly dry sample: 2637.2 g - 1232.1 g = 1405.1 g

Mass of container and sample after second drying cycle: 2634.1 g

Mass,  $M_n$ , of sample: 2634.1 g - 1232.1 g = 1402.0 g

$$\% \text{ Change} = \frac{1405.1 \text{ g} - 1402.0 \text{ g}}{1405.1 \text{ g}} \times 100 = 0.22\%$$

*0.22 percent is not less than 0.10 percent, so continue drying.*

Mass of container and sample after third drying cycle: 2633.0 g

Mass,  $M_n$ , of sample: 2633.0 g - 1232.1 g = 1400.9 g

$$\% \text{ Change} = \frac{1402.0 \text{ g} - 1400.9 \text{ g}}{1402.0 \text{ g}} \times 100 = 0.08\%$$

*0.08 percent is less than 0.10 percent, so constant mass has been reached.*

**Moisture Content Aggregate and Soils:**

Calculate the moisture content, as a percent, using the following formula:

$$w = \frac{M_W - M_D}{M_D} \times 100$$

where:

w = moisture content, percent

M<sub>W</sub> = wet mass

M<sub>D</sub> = dry mass

Example:

Mass of container: 1232.1 g

Mass of container and wet sample: 2764.7 g

Mass, M<sub>W</sub>, of wet sample: 2764.7 g - 1232.1 g = 1532.6 g

Mass of container and dry sample (COOLED): 2633.5 g

Mass, M<sub>D</sub>, of dry sample: 2633.5 g - 1232.1 g = 1401.4 g

$$w = \frac{1532.6 \text{ g} - 1401.4 \text{ g}}{1401.4 \text{ g}} \times 100 = \frac{131.2 \text{ g}}{1401.4 \text{ g}} \times 100 = 9.36\% \text{ report } 9.4\%$$

**Report**

- On forms approved by the agency
- Sample ID
- M<sub>W</sub>, wet mass
- M<sub>D</sub>, dry mass
- w, moisture content to the nearest 0.1 percent



**MOISTURE-DENSITY RELATIONS OF SOILS:  
USING A 2.5 KG (5.5 LB) RAMMER AND A 305 MM (12 IN.) DROP  
FOP FOR AASHTO T 99  
USING A 4.54 KG (10 LB) RAMMER AND A 457 MM (18 IN.) DROP  
FOP FOR AASHTO T 180**

### Scope

This procedure covers the determination of the moisture-density relations of soils and soil-aggregate mixtures in accordance with two similar test methods:

- AASHTO T 99-25: Methods A, B, C, and D
- AASHTO T 180-25: Methods A, B, C, and D

This test method applies to soil mixtures having 40 percent or less retained on the 4.75 mm (No. 4) sieve for methods A or B, or 30 percent or less retained on the 19 mm ( $\frac{3}{4}$  in.) sieve with methods C or D. The retained material is defined as oversize (coarse) material. If no minimum percentage is specified, 5 percent will be used. Samples that contain oversize (coarse) material that meet percent retained criteria should be corrected by using *Annex A, Correction of Maximum Dry Density and Optimum Moisture for Oversized Particles*. Samples of soil or soil-aggregate mixture are prepared at several moisture contents and compacted into molds of specified size, using manual or mechanical rammers that deliver a specified quantity of compactive energy. The moist masses of the compacted samples are multiplied by the appropriate factor to determine wet density values. Moisture contents of the compacted samples are determined and used to obtain the dry density values of the same samples. Maximum dry density and optimum moisture content for the soil or soil-aggregate mixture is determined by plotting the relationship between dry density and moisture content.

### Apparatus

- Mold – Cylindrical mold made of metal with the dimensions shown in Table 1 or Table 2. If permitted by the agency, the mold may be of the “split” type, consisting of two half-round sections, which can be securely locked in place to form a cylinder. Determine the mold volume according to *Annex B, Standardization of the Mold*.
- Mold assembly – Mold, base plate, and a detachable collar.
- Rammer – Manually or mechanically operated rammers as detailed in Table 1 or Table 2. A manually operated rammer shall be equipped with a guide sleeve to control the path and height of drop. The guide sleeve shall have at least four vent holes no smaller than 9.5 mm ( $\frac{3}{8}$  in.) in diameter, spaced approximately 90 degrees apart and approximately 19 mm ( $\frac{3}{4}$  in.) from each end. A mechanically operated rammer will uniformly distribute blows over the sample and will be calibrated with several soil types, and be adjusted, if necessary, to give the same moisture-density results as with the manually operated rammer. For additional information concerning calibration, see AASHTO T 99 and T 180.
- Sample extruder – A jack, lever frame, or other device for extruding compacted specimens from the mold quickly and with little disturbance.

- Balance(s) or scale(s) of the capacity and sensitivity required for the procedure used by the agency.
  - A balance or scale with a capacity of 11.5 kg (25 lb) and a sensitivity of 1 g for obtaining the sample, meeting the requirements of AASHTO M 231, Class G 5.
  - A balance or scale with a capacity of 2 kg and a sensitivity of 0.1 g is used for moisture content determinations done under both procedures, meeting the requirements of AASHTO M 231, Class G 2.
- Drying oven – Capable of maintaining a temperature of  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ) for drying moisture content samples in accordance with the FOP for AASHTO T 255/T 265.
- Straightedge – A steel straightedge at least 250 mm (10 in.) long, with one beveled edge and at least one surface plane within 0.1 percent of its length, used for final trimming.
- Sieve(s) – 4.75 mm (No. 4) and/or 19.0 mm (3/4 in.), meeting the requirements of FOP for AASHTO T 27/T 11.
- Mixing tools – Miscellaneous tools such as a mixing pan, spoon, trowel, spatula, etc., or a suitable mechanical device, for mixing the sample with water.
- Containers with close-fitting lids to prevent gain or loss of moisture in the sample.

**Table 1**  
**Comparison of Apparatus, Sample, and Procedure – Metric**

	<b>T 99</b>	<b>T 180</b>
Mold Volume, m <sup>3</sup>	Methods A, C: 0.000943 ±0.000014	Methods A, C: 0.000943 ±0.000014
	Methods B, D: 0.002124 ±0.000025	Methods B, D: 0.002124 ±0.000025
Mold Diameter, mm	Methods A, C: 101.60 ±0.40	Methods A, C: 101.60 ±0.4
	Methods B, D: 152.40 ±0.70	Methods B, D: 152.40 ±0.70
Mold Height, mm	116.40 ±0.50	116.40 ±0.50
Detachable Collar Height, mm	50.80 ±0.64	50.80 ±0.64
Rammer Diameter, mm	50.80 ±0.25	50.80 ±0.25
Rammer Mass, kg	2.495 ±0.009	4.536 ±0.009
Rammer Drop, mm	305 ±2	457 ±2
Layers	3	5
Blows per Layer	Methods A, C: 25	Methods A, C: 25
	Methods B, D: 56	Methods B, D: 56
Material Size, mm	Methods A, B: 4.75 minus	Methods A, B: 4.75 minus
	Methods C, D: 19.0 minus	Methods C, D: 19.0 minus
Test Sample Size, kg	Method A: 3 Method C: 5 (1)	Method B: 7 Method D: 11(1)
Energy, kN-m/m <sup>3</sup>	592	2,693

(1) This may not be a large enough sample depending on your nominal maximum size for moisture content samples.

**Table 2**  
**Comparison of Apparatus, Sample, and Procedure – English**

	<b>T 99</b>	<b>T 180</b>
Mold Volume, ft <sup>3</sup>	Methods A, C: 0.0333 ±0.0005	Methods A, C: 0.0333 ±0.0005
	Methods B, D: 0.07500 ±0.0009	Methods B, D: 0.07500 ±0.0009
Mold Diameter, in.	Methods A, C: 4.000 ±0.016	Methods A, C: 4.000 ±0.016
	Methods B, D: 6.000 ±0.026	Methods B, D: 6.000 ±0.026
Mold Height, in.	4.584 ±0.018	4.584 ±0.018
Detachable Collar Height, in.	2.000 ±0.025	2.000 ±0.025
Rammer Diameter, in.	2.000 ±0.025	2.000 ±0.025
Rammer Mass, lb	5.5 ±0.02	10 ±0.02
Rammer Drop, in.	12 ±0.06	18 ±0.06
Layers	3	5
Blows per Layer	Methods A, C: 25	Methods A, C: 25
	Methods B, D: 56	Methods B, D: 56
Material Size, in.	Methods A, B: No. 4 minus	Methods A, B: No.4 minus
	Methods C, D: 3/4 minus	Methods C, D: 3/4 minus
Test Sample Size, lb	Method A: 7 Method C: 12 <sub>(1)</sub>	Method B: 16 Method D: 25 <sub>(1)</sub>
Energy, lb-ft/ft <sup>3</sup>	12,375	56,250

(1) This may not be a large enough sample depending on your nominal maximum size for moisture content samples.

## Sample

If the sample is damp, dry it until it becomes friable under a trowel. Drying may be in air or by use of a drying apparatus maintained at a temperature not exceeding 60°C (140°F). Thoroughly break up aggregations in a manner that avoids reducing the natural size of individual particles.

Obtain a representative test sample of the mass required by the agency by passing the material through the sieve required by the agency. See Table 1 or Table 2 for test sample mass and material size requirements.

In instances where the material is prone to degradation, i.e., granular material, a compaction sample with differing moisture contents should be prepared for each point.

If the sample is plastic (clay types), it should stand for a minimum of 12 hours after the addition of water to allow the moisture to be absorbed. In this case, several samples at different moisture contents should be prepared, put in sealed containers, and tested the next day.

*Note 1:* Both T 99 and T 180 have four methods (A, B, C, D) that require different masses and employ different sieves.

## Procedure

During compaction, rest the mold firmly on a dense, uniform, rigid, and stable foundation, or base. This base shall remain stationary during the compaction process.

1. Determine the mass of the clean, dry mold. Include the base plate but exclude the extension collar. Record the mass to the nearest 1 g (0.005 lb).
2. Thoroughly mix the selected representative sample with sufficient water to dampen it to approximately 4 to 8 percentage points below optimum moisture content. For many materials, this condition can be identified by forming a cast by hand.
  - a. Prepare individual samples of plastic or degradable material, increasing moisture contents 1 to 2 percent for each point.
  - b. Allow samples of plastic soil to stand for **at least** 12 hrs.
3. Form a specimen by compacting the prepared soil in the mold assembly in approximately equal layers. For each layer:
  - a. Spread the loose material uniformly in the mold.

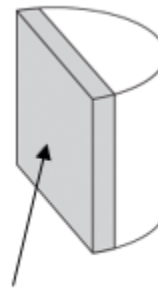
*Note 2:* It is recommended to cover the remaining material with a non-absorbent sheet or damp cloth to minimize loss of moisture.

- b. Lightly tamp the loose material with the manual rammer or other similar device, this establishes a firm surface.
- c. Compact each layer with uniformly distributed blows from the rammer. See Table 1 for mold size, number of layers, number of blows, and rammer specification for the various test methods. Use the method specified by the agency.
- d. Trim down material that has not been compacted and remains adjacent to the walls of the mold and extends above the compacted surface.

4. Remove the extension collar. Avoid shearing off the sample below the top of the mold. The material compacted in the mold should not be over 6 mm ( $\frac{1}{4}$  in.) above the top of the mold once the collar has been removed.
5. Trim the compacted soil even with the top of the mold with the beveled side of the straightedge.
6. Clean soil from exterior of the mold and base plate.
7. Determine and record the mass of the mold, base plate, and wet soil to the nearest 1 g (0.005 lb).
8. Determine and record the wet mass ( $M_w$ ) of the sample by subtracting the mass in Step 1 from the mass in Step 7.
9. Calculate the wet density ( $\rho_w$ ), in  $\text{kg/m}^3$  ( $\text{lb/ft}^3$ ), by dividing the wet mass by the measured volume ( $V_m$ ).
10. Extrude the material from the mold. For soils and soil-aggregate mixtures, slice vertically through the center and remove one of the cut faces for a representative moisture content sample. For granular materials, a vertical face will not exist. Take a representative sample ensuring that all layers are represented. This sample must meet the sample size requirements of the test method used to determine moisture content.



**Extruded material**



**Representative moisture  
content sample**

- Note 3:** When developing a curve for free-draining soils such as uniform sands and gravels, where seepage occurs at the bottom of the mold and base plate, taking a representative moisture content from the mixing bowl may be preferred in order to determine the amount of moisture available for compaction.
11. Determine and record the moisture content ( $w$ ) of the sample in accordance with the FOP for AASHTO T 255 / T 265.
  12. If the material is degradable or plastic, return to Step 3 using a prepared individual sample. If not, continue with Steps 13 through 15.
  13. Thoroughly break up the remaining portion of the molded specimen until it will again pass through the sieve, as judged by eye, and add to the remaining portion of the sample being tested.
  14. Add sufficient water to increase the moisture content of the remaining soil by 1 to 2 percentage points and repeat steps 3 through 11.

15. Continue determinations until there is either a decrease or no change in the wet mass. There will be a minimum of three points on the dry side of the curve and two points on the wet side. For non-cohesive, drainable soils, one point on the wet side is sufficient.

**Calculations**

**Wet Density**

$$\rho_w = \frac{M_w}{V_m}$$

Where:

- $\rho_w$  = wet density, kg/m<sup>3</sup> (lb/ft<sup>3</sup>)
- $M_w$  = wet mass
- $V_m$  = volume of the mold, Annex B

**Dry Density**

$$\rho_d = \left( \frac{\rho_w}{w + 100} \right) \times 100 \quad \text{or} \quad \rho_d = \frac{\rho_w}{\left( \frac{w}{100} \right) + 1}$$

Where:

- $\rho_d$  = dry density, kg/m<sup>3</sup> (lb/ft<sup>3</sup>)
- $w$  = moisture content, as a percentage

**Example for 4-inch mold, Methods A or C**

- Wet mass,  $M_w$  = 1.928 kg (4.250 lb)
- Moisture content,  $w$  = 11.3%
- Measured volume of the mold,  $V_m$  = 0.000946 m<sup>3</sup> (0.0334 ft<sup>3</sup>)

**Wet Density**

$$\rho_w = \frac{1.928 \text{ kg}}{0.000946 \text{ m}^3} = 2038 \text{ kg/m}^3 \quad \rho_w = \frac{4.250 \text{ lb}}{0.0334 \text{ ft}^3} = 127.2 \text{ lb/ft}^3$$

Where:

- $\rho_w$  = Wet density, kg/m<sup>3</sup> (lb/ft<sup>3</sup>)

**Dry Density**

$$\rho_d = \left( \frac{2038 \text{ kg/m}^3}{11.3 + 100} \right) \times 100 = 1831 \text{ kg/m}^3 \quad \rho_d = \left( \frac{127.2 \text{ lb/ft}^3}{11.3 + 100} \right) \times 100 = 114.3 \text{ lb/ft}^3$$

Or

$$\rho_d = \left( \frac{2038 \text{ kg/m}^3}{\frac{11.3}{100} + 1} \right) = 1831 \text{ kg/m}^3 \quad \rho_d = \left( \frac{127.2 \text{ lb/ft}^3}{\frac{11.3}{100} + 1} \right) = 114.3 \text{ lb/ft}^3$$

$$\rho_d = \text{Dry density, kg/m}^3 \text{ (lb/ft}^3\text{)}$$

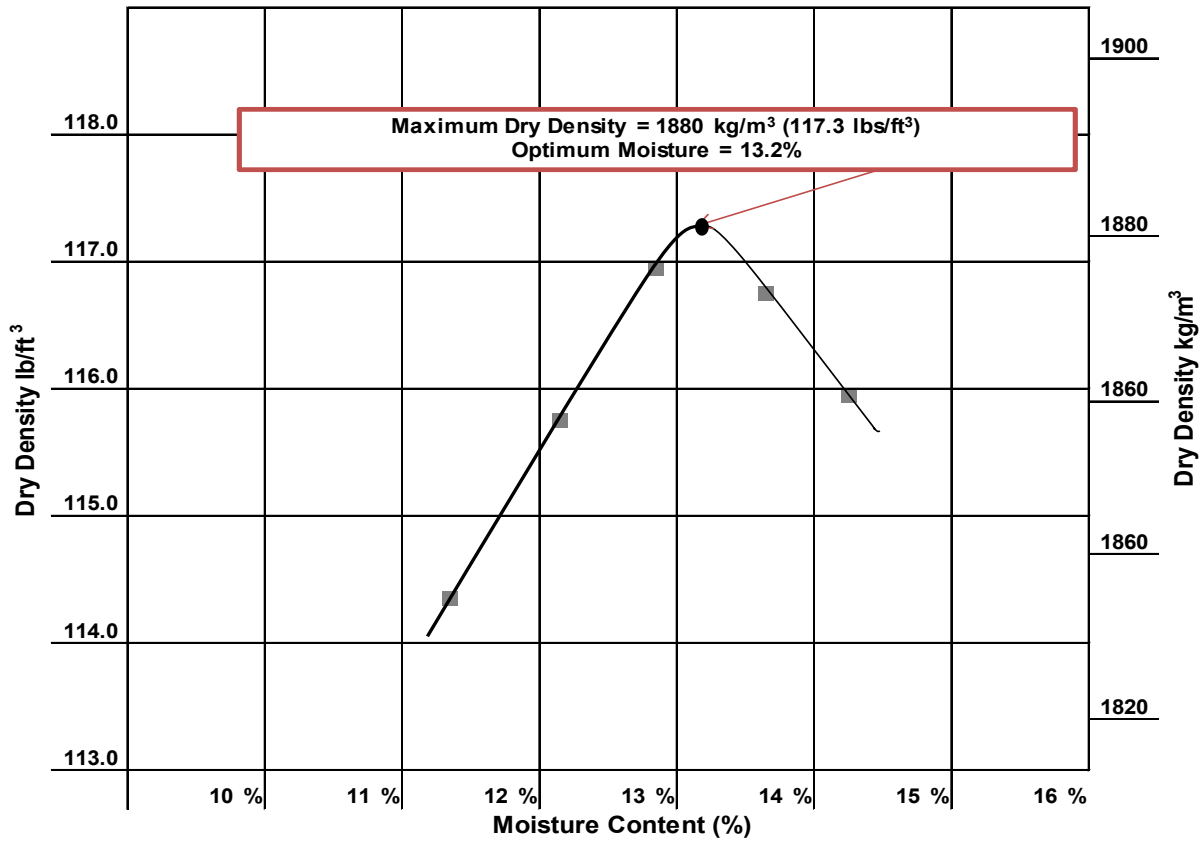
**Moisture-Density Curve Development**

When dry density is plotted on the vertical axis versus moisture content on the horizontal axis and the points are connected with a smooth line, a moisture-density curve is developed. The coordinates of the peak of the curve are the maximum dry density, or just “maximum density,” and the “optimum moisture content” of the soil.

**Example**

Given the following dry density and corresponding moisture content values develop a moisture-density relations curve and determine maximum dry density and optimum moisture content.

Dry Density		Moisture Content, %
kg/m <sup>3</sup>	lb/ft <sup>3</sup>	
1831	114.3	11.3
1853	115.7	12.1
1873	116.9	12.8
1869	116.7	13.6
1857	115.9	14.2



In this case, the curve has its peak at:

Maximum dry density = 1880 kg/m<sup>3</sup> (117.3 lb/ft<sup>3</sup>)  
Optimum moisture content = 13.2%

Note that both values are approximate since they are based on sketching the curve to fit the points.

### Report

- Results on forms approved by the agency
- Sample ID
- Maximum dry density to the nearest 1 kg/m<sup>3</sup> (0.1 lb/ft<sup>3</sup>)
- Optimum moisture content to the nearest 0.1 percent

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**ANNEX A****CORRECTION OF MAXIMUM DRY DENSITY AND OPTIMUM MOISTURE FOR OVERSIZED PARTICLES**

(Mandatory Information)

This section corrects the maximum dry density and moisture content of the material retained on the 4.75 mm (No. 4) sieve, Methods A and B; or the material retained on the 19 mm (¾ in.) sieve, Methods C and D. The maximum dry density, corrected for oversized particles and total moisture content, are compared with the field-dry density and field moisture content.

This correction can be applied to the sample on which the maximum dry density is performed. A correction may not be practical for soils with only a small percentage of oversize material. The agency shall specify a minimum percentage below which the method is not needed. If not specified, this method applies when more than 5 percent by weight of oversize particles is present.

Bulk specific gravity ( $G_{sb}$ ) of the oversized particles is required to determine the corrected maximum dry density. Use the bulk specific gravity as determined using the FOP for AASHTO T 85 in the calculations. For construction activities, an agency established value or specific gravity of 2.600 may be used.

This correction can also be applied to the sample obtained from the field while performing in-place density.

**Procedure**

1. Use the sample from this procedure or a sample obtained according to the FOP for AASHTO T 310.
2. Sieve the sample on the 4.75 mm (No. 4) sieve for Methods A and B or the 19 mm (¾ in.) sieve, Methods C and D.
3. Determine the dry mass of the oversized and fine fractions ( $M_{DC}$  and  $M_{DF}$ ) by one of the following:
  - a. Dry the fractions, fine and oversized, in air or by use of a drying apparatus that is maintained at a temperature not exceeding 60°C (140°F).
  - b. Calculate the dry masses using the moisture samples.

To determine the dry mass of the fractions using moisture samples.

1. Determine the moist mass of both fractions, fine ( $M_{Mf}$ ) and oversized ( $M_{Mc}$ ):
2. Obtain moisture samples from the fine and oversized material.
3. Determine the moisture content of the fine particles ( $MC_f$ ) and oversized particles ( $MC_c$ ) of the material by FOP for AASHTO T 255/T 265 or agency approved method.
4. Calculate the dry mass of the oversize and fine particles.

$$M_D = \frac{M_m}{1 + MC}$$

Where:

$M_D$  = mass of dry material (fine or oversize particles)

$M_m$  = mass of moist material (fine or oversize particles)

MC = moisture content of respective fine or oversized, expressed as a decimal

- Calculate the percentage of the fine ( $P_f$ ) and oversized ( $P_c$ ) particles by dry weight of the total sample as follows: See Note 2.

$$P_f = \frac{100 \times M_{DF}}{M_{DF} + M_{DC}} \quad \frac{100 \times 15.4 \text{ lb}}{15.4 \text{ lbs} + 5.7 \text{ lb}} = 73\% \quad \frac{100 \times 6.985 \text{ kg}}{6.985 \text{ kg} + 2.585 \text{ kg}} = 73\%$$

And

$$P_c = \frac{100 \times M_{DC}}{M_{DF} + M_{DC}} \quad \frac{100 \times 5.7 \text{ lb}}{15.4 \text{ lbs} + 5.7 \text{ lb}} = 27\% \quad \frac{100 \times 2.585 \text{ kg}}{6.985 \text{ kg} + 2.585 \text{ kg}} = 27\%$$

Or for  $P_c$ :

$$P_c = 100 - P_f$$

Where:

$P_f$  = percent of fine particles, of sieve used, by weight

$P_c$  = percent of oversize particles, of sieve used, by weight

$M_{DF}$  = mass of dry fine particles

$M_{DC}$  = mass of dry oversize particles

### Optimum Moisture Correction Equation

1. Calculate the corrected moisture content as follows:

$$MC_T = \frac{(MC_F \times P_f) + (MC_C \times P_c)}{100} = \frac{(13.2\% \times 73.0\%) + (2.1\% \times 27.0\%)}{100} = 10.2\%$$

$MC_T$  = corrected moisture content of combined fines and oversized particles, expressed as a % moisture

$MC_F$  = moisture content of fine particles, as a % moisture

$MC_C$  = moisture content of oversized particles, as a % moisture

**Note 1:** Moisture content of oversize material can be assumed to be two (2) percent for most construction applications.

**Note 2:** In some field applications agencies will allow the percentages of oversize and fine materials to be determined with the materials in the wet state.

### Density Correction Equation

2. Calculate the corrected dry density ( $\rho_d$ ) of the total sample (combined fine and oversized particles) as follows:

$$\rho_d = \frac{100\%}{\left[\left(\frac{P_f}{\rho_f}\right) + \left(\frac{P_c}{k}\right)\right]}$$

Where:

$\rho_d$  = corrected total dry density (combined fine and oversized particles)  
kg/m<sup>3</sup> (lb/ft<sup>3</sup>)

$\rho_f$  = dry density of the fine particles kg/m<sup>3</sup> (lb/ft<sup>3</sup>), determined in the lab

$P_c$  = percent of dry oversize particles, of sieve used, by weight.

$P_f$  = percent of dry fine particles, of sieve used, by weight.

$k$  = Metric: 1,000 \* Bulk Specific Gravity ( $G_{sb}$ ) (oven dry basis)  
of coarse particles (kg/m<sup>3</sup>).

$k$  = English: 62.4 \* Bulk Specific Gravity ( $G_{sb}$ ) (oven dry basis)  
of coarse particles (lb/ft<sup>3</sup>)

**Note 3:** If the specific gravity is known, then this value will be used in the calculation. For most construction activities the specific gravity for aggregate may be assumed to be 2.600.

**Calculation**

**Example**

- Metric:

Maximum laboratory dry density ( $\rho_p$ ):	1880 kg/m <sup>3</sup>
Percent coarse particles ( $P_c$ ):	27%
Percent fine particles ( $P_f$ ):	73%
Mass per volume coarse particles ( $k$ ):	(2.697) (1000) = 2697 kg/m <sup>3</sup>

$$\rho_d = \frac{100\%}{\left[\left(\frac{P_f}{\rho_f}\right) + \left(\frac{P_c}{k}\right)\right]}$$

$$\rho_d = \frac{100\%}{\left[\left(\frac{73\%}{1880 \text{ kg/m}^3}\right) + \left(\frac{27\%}{2697 \text{ kg/m}^3}\right)\right]}$$

$$\rho_d = \frac{100\%}{[0.03883 \text{ kg/m}^3 + 0.01001 \text{ kg/m}^3]}$$

$$\rho_d = 2047.5 \text{ kg/m}^3 \text{ report } 2048 \text{ kg/m}^3$$

English:

Maximum laboratory dry density ( $\rho_f$ ): 117.3 lb/ft<sup>3</sup>

Percent coarse particles ( $P_c$ ): 27%

Percent fine particles ( $P_f$ ): 73%

Mass per volume of coarse particles (k): (2.697) (62.4) = 168.3 lb/ft<sup>3</sup>

$$\rho_d = \frac{100\%}{\left[\left(\frac{P_f}{\rho_f}\right) + \left(\frac{P_c}{k}\right)\right]}$$

$$\rho_d = \frac{100\%}{\left[\left(\frac{73\%}{117.3 \text{ lb/ft}^3}\right) + \left(\frac{27\%}{168.3 \text{ lb/ft}^3}\right)\right]}$$

$$\rho_d = \frac{100\%}{[0.6223 \text{ lb/ft}^3 + 0.1604 \text{ lb/ft}^3]}$$

$$\rho_d = \frac{100\%}{0.7827 \text{ lb/ft}^3}$$

$$\rho_d = 127.76 \text{ lb/ft}^3 \quad \text{Report } 127.8 \text{ lb/ft}^3$$

### Report

- On forms approved by the agency
- Sample ID
- Corrected maximum dry density to the nearest 1 kg/m<sup>3</sup> (0.1 lb/ft<sup>3</sup>)
- Corrected optimum moisture to the nearest 0.1 percent

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## ANNEX B

### STANDARDIZATION OF THE MOLD

(Mandatory Information)

Standardization is a critical step to ensure accurate test results when using this apparatus. Failure to perform the standardization procedure as described herein will produce inaccurate or unreliable test results.

#### Apparatus

- Mold and base plate
- Balance or scale – Accurate to within 45 g (0.1 lb) or 0.3 percent of the test load, whichever is greater, at any point within the range of use.
- Cover plate – A piece of plate glass, at least 6 mm (1/4 in.) thick and at least 25 mm (1 in.) larger than the diameter of the mold.
- Thermometers – Standardized liquid-in-glass, or electronic digital total immersion type, accurate to 0.5°C (1°F)

#### Procedure

1. Create a watertight seal between the mold and base plate.
2. Determine and record the mass of the dry sealed mold, base plate, and cover plate.
3. Fill the mold with water at a temperature between 16°C and 29°C (60°F and 85°F) and cover with the cover plate in such a way as to eliminate bubbles and excess water.
4. Wipe the outside of the mold, base plate, and cover plate dry, being careful not to lose any water from the mold.
5. Determine and record the mass of the filled mold, base plate, cover plate, and water.
6. Determine and record the mass of the water in the mold by subtracting the mass in Step 2 from the mass in Step 5.
7. Measure the temperature of the water and determine its density from Table B1, interpolating, as necessary.
8. Calculate the volume of the mold,  $V_m$ , by dividing the mass of the water in the mold by the density of the water at the measured temperature.

**Calculations**

$$V_m = \frac{M}{\rho_{water}}$$

Where:

$V_m$  = volume of the mold

$M$  = mass of water in the mold

$\rho_{water}$  = density of water at the measured temperature

**Example**

Mass of water in mold = 0.94367 kg (2.0800 lb)

$\rho_{water}$  at 23°C (73.4°F) = 997.54 kg/m<sup>3</sup> (62.274 lb/ft<sup>3</sup>)

$$V_m = \frac{0.94367 \text{ kg}}{997.54 \text{ kg/m}^3} = 0.000946 \text{ m}^3 \quad V_m = \frac{2.0800 \text{ lb}}{62.274 \text{ lb/ft}^3} = 0.0334 \text{ ft}^3$$

**Table B1  
Unit Mass of Water  
15°C to 30°C**

°C	(°F)	kg/m <sup>3</sup>	(lb/ft <sup>3</sup> )	°C	(°F)	kg/m <sup>3</sup>	(lb/ft <sup>3</sup> )
15	(59.0)	999.10	(62.372)	23	(73.4)	997.54	(62.274)
15.6	(60.0)	999.01	(62.366)	23.9	(75.0)	997.32	(62.261)
16	(60.8)	998.94	(62.361)	24	(75.2)	997.29	(62.259)
17	(62.6)	998.77	(62.350)	25	(77.0)	997.03	(62.243)
18	(64.4)	998.60	(62.340)	26	(78.8)	996.77	(62.227)
18.3	(65.0)	998.54	(62.336)	26.7	(80.0)	996.59	(62.216)
19	(66.2)	998.40	(62.328)	27	(80.6)	996.50	(62.209)
20	(68.0)	998.20	(62.315)	28	(82.4)	996.23	(62.192)
21	(69.8)	997.99	(62.302)	29	(84.2)	995.95	(62.175)
21.1	(70.0)	997.97	(62.301)	29.4	(85.0)	995.83	(62.166)
22	(71.6)	997.77	(62.288)	30	(86.0)	995.65	(62.156)

**Report**

- Mold ID
- Date Standardized
- Temperature of the water
- Volume,  $V_m$ , of the mold to the nearest  $0.000001 \text{ m}^3$  ( $0.0001 \text{ ft}^3$ )

## DEVELOPING SOIL MOISTURE-DENSITY RELATIONS (FAMILY OF CURVES) FOP FOR AASHTO R 75

### Scope

This procedure provides a method to develop soils density-relations in accordance with AASHTO R 75-24 using multiple moisture-density curves developed using the same method, A, B, C, or D, from the FOP for AASHTO T 99/T 180.

All curves used in a group must be developed using a single Method: A, B, C, or D of a procedure for AASHTO T 99 or T 180. See the FOP for AASHTO T 99/T 180.

### Terminology

*soil moisture-density relations* — a group of soil moisture-density relations (commonly known as a family of curves) determined using AASHTO T 99 or T 180, which reveal certain similarities and trends characteristic of the soil type and source.

*spine* — smooth line extending through the point of maximum density and optimum moisture content of a group of moisture-density curves.

### Procedure

1. Sort the curves by Method (A, B, C, or D of the FOP for T 99/T 180). At least three curves are required per group.
2. Select the highest and lowest maximum dry densities from those selected to assist in determining the desired scale of the subsequent graph.
3. Plot the maximum density and optimum moisture points of the selected curves on the graph.
4. Draw a smooth, “best fit,” curved line through the points creating the spine of the soil moisture-density relations.
5. Remove maximum density and optimum moisture points that were not used to establish the spine.
6. Add the moisture-density curves associated with the points that were used to establish the spine. It is not necessary to include the portion of the curves over optimum moisture.

*Note 1*—Intermediate curves using slopes similar to those of the original moisture-density curves may be included when maximum density points are more than 2.0 lb/ft<sup>3</sup> apart. Intermediate curves are indicated by a dashed line.

7. Plot the 80 percent of optimum moisture range when desired:
  - a. Using the optimum moisture of an existing curve, calculate 80 percent of optimum moisture and plot this value on the curve. Repeat for each curve in the group.
  - b. Draw a smooth, “best fit,” curved line connecting the 80 percent of optimum moisture points plotted on the curves that parallel the spine.

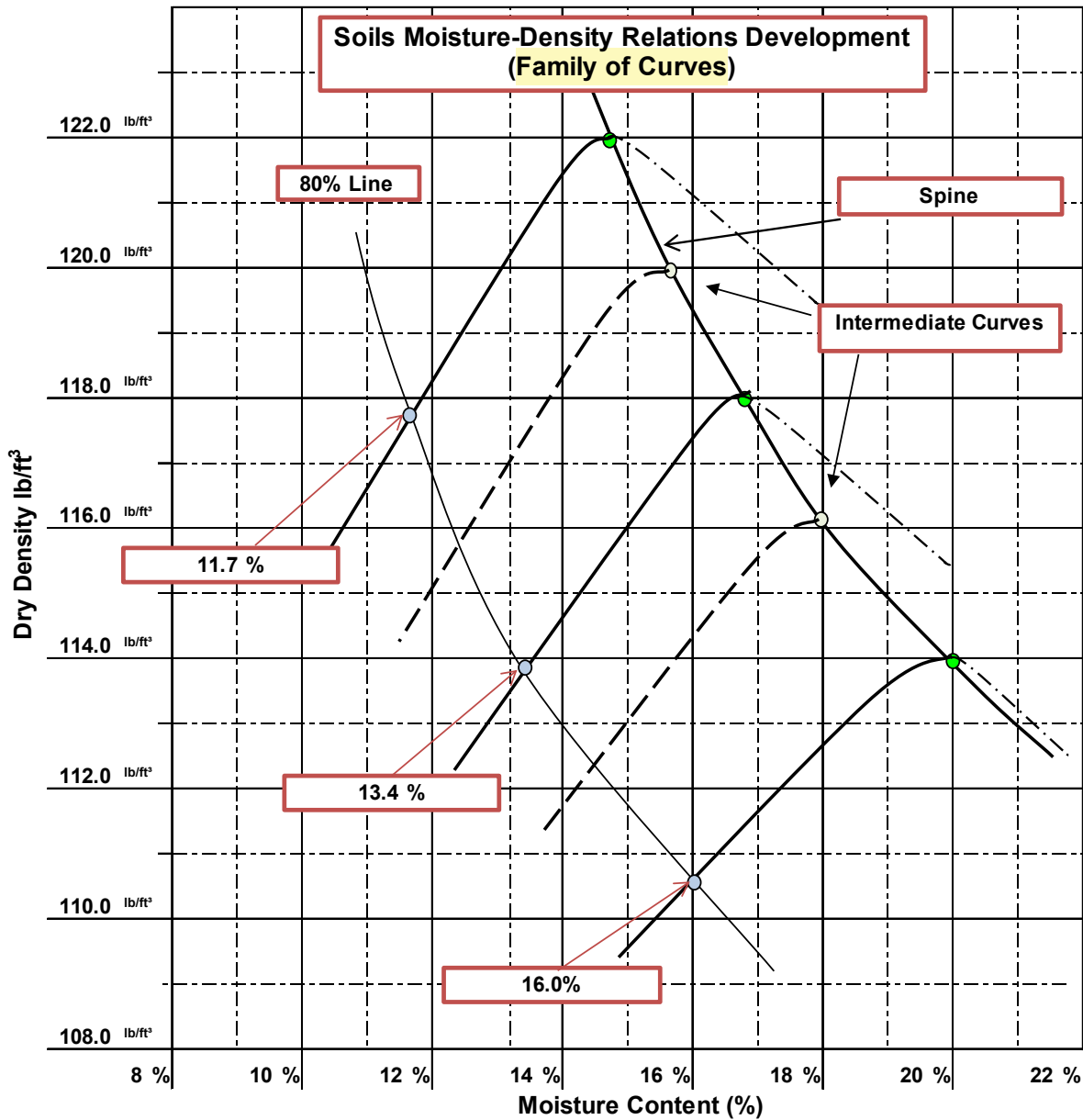
### Calculations

Calculate 80 percent of optimum moisture of each curve:

Example:

Optimum moisture of the highest density curve = 14.6%

$$80\% \text{ point} = \frac{80}{100} \times 14.6\% = 11.7\%$$



## SPECIFIC GRAVITY AND ABSORPTION OF COARSE AGGREGATE FOP FOR AASHTO T 85

### Scope

This procedure covers the determination of specific gravity and absorption of coarse aggregate in accordance with AASHTO T 85-25. Specific gravity may be expressed as bulk specific gravity ( $G_{sb}$ ), bulk specific gravity, saturated surface dry ( $G_{sb}$  SSD), or apparent specific gravity ( $G_{sa}$ ).  $G_{sb}$  and absorption are based on aggregate after soaking in water. This procedure is not intended to be used with lightweight aggregates.

### Terminology

Absorption – the increase in the mass of aggregate due to water being absorbed into the pores of the material but not including water adhering to the outside surface of the particles, expressed as a percentage of the dry mass.

Saturated Surface Dry (SSD) – condition of an aggregate particle when the permeable voids are filled with water, but no water is present on exposed surfaces.

Specific Gravity – the ratio of the mass, in air, of a volume of a material to the mass of the same volume of gas-free distilled water at a stated temperature.

Apparent Specific Gravity ( $G_{sa}$ ) – the ratio of the mass, in air, of a volume of the impermeable portion of aggregate to the mass of an equal volume of gas-free distilled water at a stated temperature.

Bulk Specific Gravity ( $G_{sb}$ ) – the ratio of the mass, in air, of a volume of aggregate (including the permeable and impermeable voids in the particles but not including the voids between particles) to the mass of an equal volume of gas-free distilled water at a stated temperature.

Bulk Specific Gravity (SSD) ( $G_{sb}$  SSD) – the ratio of the mass, in air, of a volume of aggregate, including the mass of water within the voids filled to the extent achieved by submerging in water for 15 to 19 hours (but not including the voids between particles), to the mass of an equal volume of gas-free distilled water at a stated temperature.

### Apparatus

- Balance or scale: with a capacity of 5 kg, sensitive to 0.1 g. Meeting the requirements of AASHTO M 231.
- Sample container: a wire basket of 3.35 mm (No. 6) or smaller mesh, with a capacity of 4 to 7 L (1 to 2 gal) to contain aggregate with a nominal maximum size of 37.5 mm (1 1/2 in.) or smaller; or a larger basket for larger aggregates, or both.
- Water tank: watertight and large enough to completely immerse aggregate and basket, equipped with an overflow valve to keep water level constant.
- Suspension apparatus: wire used to suspend apparatus shall be of the smallest practical diameter.

- Sieves: 4.75 mm (No. 4) or other sizes as needed, meeting the requirements of FOP for AASHTO T 27/T 11.
- Large absorbent cloth

**Sample Preparation**

1. Obtain the sample in accordance with the FOP for AASHTO R 90 (see Note 1).
2. Mix the sample thoroughly and reduce it to the approximate sample size required by Table 1 in accordance with the FOP for AASHTO R 76.
3. Reject all material passing the appropriate sieve by dry sieving.
4. Thoroughly wash the sample to remove dust or other coatings from the surface.
5. Dry the sample to constant mass according to the FOP for AASHTO T 255/T 265 at a temperature of 110 ±5°C (230 ±9°F) and cool in air at room temperature for 1 to 3 hours.

*Note 1:* Where the absorption and specific gravity values are to be used in proportioning concrete mixtures in which the aggregates will be in their naturally moist condition, the requirement for initial drying to constant mass may be eliminated, and, if the surfaces of the particles in the sample have been kept continuously wet until test, the 15-to-19-hour soaking may also be eliminated.

6. Re-screen the sample over the appropriate sieve. Reject all material passing that sieve.
7. The sample shall meet or exceed the minimum mass given in Table 1.

*Note 2:* If this procedure is used only to determine the G<sub>sb</sub> of oversized material for the FOP for AASHTO T 99 / T 180, the material can be rejected over the appropriate sieve. For T 99 / T 180 Methods A and B, use the 4.75 mm (No. 4) sieve; T 99 / T 180 Methods C and D use the 19 mm (3/4 in).

**Table 1**

Nominal Maximum Size* mm (in.)	Minimum Mass of Sample, g (lb)
12.5 (1/2) or less	2000 (4.4)
19.0 (3/4)	3000 (6.6)
25.0 (1)	4000 (8.8)
37.5 (1 1/2)	5000 (11)
50 (2)	8000 (18)
63 (2 1/2)	12,000 (26)
75 (3)	18,000 (40)

\* One sieve larger than the first sieve to retain more than 10 percent of the material using an agency specified set of sieves based on cumulative percent retained. Where large gaps in specification sieves exist, intermediate sieve(s) may be inserted to determine nominal maximum size.

**Procedure**

1. Immerse the sample in water at room temperature for a period of 15 to 19 hours.

*Note 3:* When testing coarse aggregate of large nominal maximum size requiring large samples, it may be more convenient to perform the test on two or more subsamples, and then combine the values obtained.

2. Place the empty basket into the water bath and attach to the balance. Inspect the immersion tank to ensure the water level is at the overflow outlet height and basket is fully submerged. Tare the balance with the empty basket attached in the water bath.
3. Remove the sample from the water and roll it in a large absorbent cloth until all visible films of water are removed. Wipe the larger particles individually. If the sample dries past the SSD condition, immerse in water for 30 min, and then resume the process of surface-drying.

*Note 4:* A moving stream of air may be used to assist in the drying operation but take care to avoid evaporation of water from aggregate pores.

4. Determine the SSD mass of the sample, and record this and all subsequent masses to the nearest 0.1 g or 0.1 percent of the sample mass, whichever is greater. Designate this mass as “B.”
5. Immediately place the SSD sample in the sample container and weigh it in water maintained at  $23.0 \pm 1.7^{\circ}\text{C}$  ( $73.4 \pm 3^{\circ}\text{F}$ ). Shake the container to release entrapped air before recording the weight. Re-inspect the immersion tank to ensure the water level is at the overflow outlet height and basket is fully submerged. Designate this submerged weight as “C.”

*Note 5:* The container should be immersed to a depth sufficient to cover it and the sample during mass determination. Wire suspending the container should be of the smallest practical size to minimize any possible effects of a variable immersed length.

6. Remove the sample from the basket. Ensure all material has been removed. Place in a container of known mass.
7. Dry the sample to constant mass according to the FOP for AASHTO T 255 / T 265 at  $110 \pm 5^{\circ}\text{C}$  ( $230 \pm 9^{\circ}\text{F}$ ) and cool in air at room temperature for 1 to 3 hours.
8. Determine and record the dry mass. Designate this mass as “A.”

**Calculations**

Perform calculations and determine values using the appropriate formula below.

Bulk specific gravity ( $G_{sb}$ )

$$G_{sb} = \frac{A}{B - C}$$

Bulk specific gravity, SSD ( $G_{sb}$  SSD)

$$G_{sb}SSD = \frac{B}{B - C}$$

Apparent specific gravity ( $G_{sa}$ )

$$G_{sa} = \frac{A}{A - C}$$

Absorption

$$\text{Absorption} = \frac{B - A}{A} \times 100$$

Where:

- A = oven dry mass, g
- B = SSD mass, g
- C = weight in water, g

**Sample Calculations**

Sample	A	B	C	B - C	A - C	B - A
1	2030.9	2044.9	1304.3	740.6	726.6	14.0
2	1820.0	1832.5	1168.1	664.4	651.9	12.5
3	2035.2	2049.4	1303.9	745.5	731.3	14.2

Sample	G <sub>sb</sub>	G <sub>sb</sub> SSD	G <sub>sa</sub>	Absorption
1	2.742	2.761	2.795	0.7
2	2.739	2.758	2.792	0.7
3	2.730	2.749	2.783	0.7

These calculations demonstrate the relationship between G<sub>sb</sub>, G<sub>sb</sub> SSD, and G<sub>sa</sub>. G<sub>sb</sub> is always lowest since the volume includes voids permeable to water. G<sub>sb</sub> SSD is always intermediate. G<sub>sa</sub> is always highest since the volume does not include voids permeable to water. When running this test, check to make sure the values calculated make sense in relation to one another.

**Report**

- On forms approved by the agency
- Sample ID
- Specific gravity values to the nearest 0.001
- Absorption to the nearest 0.1 percent

